

Chapter 16A - Structural Design

2001 CBC	PROPOSED ADOPTION	OSHPD		DSA-SS	Comments
		1	4		
	Adopt entire chapter without amendments				
	Adopt entire chapter with amendments listed below	X	X	X	
	Adopt only those sections listed below				
	<i>1601A.1.1 CA</i>	X	X	X	
	<i>1601A.1.2 CA</i>	X	X	X	
	<i>1601A.2 CA</i>	X	X	X	
	<i>1601A.3 CA</i>	X	X	X	
<i>1641A CA, 1602A</i>	<i>1602A.1</i>	X	X	X	Relocated existing California Building Standards into IBC format
	<i>1603A.1</i>	X	X	X	
<i>1633A.2.3</i>	<i>1603A.1.5.1 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1614A.1 CA</i>	<i>1603A.3.1 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1607A.3.5.2 CA</i>	<i>1603A.3.2 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>Table 16A-W</i>	<i>Table 1604A.3</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1613A.2 CA</i>	<i>1604A.3.7 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1613A.2.1 CA</i>	<i>1604A.3.7.1 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1613A.2.3 CA</i>	<i>1604A.3.7.2 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>1613A.1</i>	<i>1604A.3.8 CA</i>	X	X	X	Relocated existing California Building Standards into IBC format
<i>Table 16A-K</i>	<i>Table 1604A.5</i>	X	X	X	Relocated existing California Building Standards into IBC format

1605A.5 CA	1604A.11 CA	X	X	X	Relocated existing California Building Standards into IBC format
	1605A.2.1.1			X	
1632A.1	1605A.3.2	X	X	X	Relocated existing California Building Standards into IBC format
	1605A.5	X	X	X	
1607A.4.1	1606A.3 CA	X	X	X	Relocated existing California Building Standards into IBC format
Table 16A-A, 16A-B	Table 1607A.1	X	X	X	Relocated existing California Building Standards into IBC format
1607A.4.4	1607A.11.2.2	X	X	X	Relocated existing California Building Standards into IBC format
1611A.5	1607A.13	X	X	X	Relocated existing California Building Standards into IBC format
	1608A.2	X	X	X	
	Table 1608.2				Deleted
	1609A.1.1.2 CA			X	
1620A	1609A.1.3 CA	X	X	X	Relocated existing California Building Standards into IBC format
1619A	1609A.4	X	X	X	Relocated existing California Building Standards into IBC format
	1612A.3	X	X	X	
	1612A.5	X	X	X	Editorial
	1613A.1	X	X	X	
1626A.4 CA	1613A.1.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
1627A & 1641A CA	1613A.2	X	X	X	Relocated existing California Building Standards into IBC format
	1613A.5.1	X	X	X	
	FIGURES 1613.5 (1) – (14)	X	X	X	Use Figures in Chapter 16
	1613A.5.6	X	X	X	
	Table 1613.5.6 (1)				Deleted
	Table 1613.5.6(2)				Deleted
	1613A.5.6.1	X	X	X	

1630A.2.3	1613A.5.6.2	X	X	X	Relocated existing California Building Standards into IBC format
	1614A CA	X	X	X	Modifications to ASCE 7
1629A.8.3,16 30A.4.2	1614A.1.4 CA	X	X	X	Relocated existing California Building Standards into IBC format
1629A.9.1, 1629A.9.4 CA, 1629A.9.5 CA	1614A.1.5 CA	X	X	X	Relocated existing California Building Standards into IBC format
1630A.1.1	1614A.1.6 CA	X	X	X	Relocated existing California Building Standards into IBC format
1631A.5.4	1614A.1.9 CA	X	X	X	Relocated existing California Building Standards into IBC format
1633A.2.12	1614A.1.10 CA	X	X	X	Relocated existing California Building Standards into IBC format
2501A.5 CA,	1614A.1.12 CA	X	X	X	Relocated existing California Building Standards into IBC format
1632A.6 CA	1614A.1.13 CA	X	X	X	Relocated existing California Building Standards into IBC format
1633A.2.13.1 CA	1614A.1.15 CA	X	X	X	Relocated existing California Building Standards into IBC format
1633A.2.13.1 CA	1614A.1.16 CA	X	X	X	Relocated existing California Building Standards into IBC format
1657A.3	1614A.1.18 CA	X	X	X	Relocated existing California Building Standards into IBC format
1661A.2.7	1614A.1.19 CA	X	X	X	Relocated existing California Building Standards into IBC format
1661A.2.8, Item #3 & #6	1614A.1.20 CA	X	X	X	Relocated existing California Building Standards into IBC format
1661A.2.9	1614A.1.21 CA	X	X	X	Relocated existing California Building Standards into IBC format
1661A.2.8, Item #5	1614A.1.22 CA	X	X	X	Relocated existing California Building Standards into IBC format
1661A.3.2	1614A.1.23 CA	X	X	X	Relocated existing California Building Standards into IBC

					format
1657A.5.3.3	1614A.1.23 CA	X	X	X	Relocated existing California Building Standards into IBC format
1659A.4.2	1614A.1.24 CA	X	X	X	Relocated existing California Building Standards into IBC format
1657A.5.2	1614A.1.25 CA	X	X	X	Relocated existing California Building Standards into IBC format
1657A.5.3	1614A.1.26 CA	X	X	X	Relocated existing California Building Standards into IBC format
1657A.5.1.1 CA	1614A.1.27 CA	X	X	X	Relocated existing California Building Standards into IBC format
1664A.1 CA	1614A.1.28 CA	X	X	X	Relocated existing California Building Standards into IBC format

REPEAL OF EXISTING CALIFORNIA AMENDMENTS IN PART OR IN WHOLE THAT ARE NO LONGER NECESSARY AS FOLLOWS:

2001 CBC DIVISION I – GENERAL DESIGN REQUIREMENTS

2001 CBC SECTION 1605A – DESIGN: Repeal amendments in following subsections.

~~1601A.2, 1601A.2.1, 1601A.3 and 1605A.4.~~

2001 CBC SECTION 1607A – LIVE LOADS: Repeal amendments in following subsections.

~~1607A.3.3, 1607A.3.5 and 1607A.6.~~

2001 CBC SECTION 1611A – OTHER MINIMUM LOADS: Repeal amendments in following subsections.

~~1611A.11.3, 1611A.12, 1611A.12.1 And 1611A.12.2.~~

2001 CBC SECTION 1612A – COMBINATION OF LOADS: Repeal amendments in following subsections.

~~1612A.2.1, 1612A.3.1 and 1612A.3.2.~~

2001 CBC SECTION 1613A – DEFLECTION: Repeal amendments in following subsections.

~~1613A.2.2.1, 1613A.2.2.2, and 1613A.2.2.3.~~

2001 CBC DIVISION II – SNOW LOADS

2001 CBC SECTION 1614A – SNOW LOADS: Repeal amendment in the following subsection.

~~1614A.~~

2001 CBC DIVISION III – WIND DESIGN

~~2001 CBC SECTION 1615A – GENERAL:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1616A – DEFINITIONS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1621A – PRIMARY FRAMES AND SYSTEMS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1622A – ELEMENTS AND COMPONENTS OF STRUCTURES:~~ Repeal all amendments in this section.

2001 CBC DIVISION IV – EARTHQUAKE DESIGN

2001 CBC SECTION 1626A – GENERAL: Repeal amendments in following subsections.

~~1626A.1~~ and ~~1626A.2~~.

~~2001 CBC SECTION 1627A – DEFINITIONS:~~ Repeal all amendments in this section.

2001 CBC SECTION 1629A – CRITERIA SELECTION: Repeal amendments in following subsections.

~~1629A.4.1, 1629A.5.3, 1629A.6.5, 1629A.8.3, 1629A.8.4, 1629A.9.1, 1629A.9.4, 1629A.5, 1629A.10.1 and 1629A.10.2.~~

2001 CBC SECTION 1630A – Minimum Design Lateral Forces and Related Effects: Repeal amendments in following subsections.

~~1630A.1.1~~ except item # 5, ~~1630A.1.2, 1630A.2.2, 1630A.7 and 1630A.10.2.~~

2001 CBC SECTION 1631A – DYNAMIC ANALYSIS PROCEDURE: Repeal amendments in following subsections.

~~1631A.2, 1631A.3, 1631A.5.4 and 631A.6.3.2 .~~

2001 CBC SECTION 1632A – LATERAL FORCE ON ELEMENTS OF STRUCTURES, NONSTRUCTURAL COMPONENTS AND EQUIPMENT SUPPORTED BY STRUCTURES: Repeal amendment in the following subsection.

~~1632A.1, 1632A.2 and 1632A.6.~~

2001 CBC SECTION 1633A – DETAILED SYSTEM DESIGN REQUIREMENTS: Repeal amendments in following subsections.

~~1633A.1, 1633A.2.7, 1633A.2.9 and 1633A.2.13.~~

~~2001 CBC SECTION 1635A – EARTHQUAKE RECORDING INSTRUMENTATIONS:~~ Repeal all amendments in this section.

2001 CBC CHAPTER 16A – TABLES AND FIGURES: Repeal amendments in following tables and figure.

~~Tables 16A-A, 16A-B, 16A-D, 16A-E, 16A-H, 16A-I, 16A-K, 16A-L, 16A-M, 16A-N, 16A-O, 16A-P, 16A-V, 16-W and Figure 16A-2.~~

2001 CBC DIVISION VI-R – EARTHQUAKE EVALUATION AND DESIGN OF EXISTING HOSPITAL BUILDINGS.

2001 CBC SECTION 1640A – GENERAL: Repeal amendments in following subsections.

~~1640A.4, 1640A.5, 1640A.6, 1640A.8.1, 1640A.8.2, and 1640A.8.3~~

~~2001 CBC SECTION 1641A – DEFINITIONS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1642A – SYMBOLS AND NOTATIONS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1643A – CRITERIA SELECTION :~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1644A – METHOD A:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1645A – PROCEDURES FOR THE CLASSIFICATION OF ELEMENTS INTO THE DUCTILE, LIMITED DUCTILE AND NONDUCTILE CATEGORIES:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1646A – DETAILED SYSTEM DESIGN REQUIREMENTS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1647A – NONBUILDING STRUCTURES:~~ Repeal all amendments in this section.

2001 CBC SECTION 1648A – METHOD B: Repeal amendments in following subsections.

~~1648A.2~~ including all subsections and ~~1648A.3~~.

2001 CBC CHAPTER 16A – TABLES AND FIGURES: Repeal amendments in following tables and figures.

~~TABLES 16-R-1, 16-R-2, 16-R-3, 16-R-4, 16-R-5, 16-R-1 and Figure 16-R-2.~~

2001 CBC APPENDIX CHAPTER 16A

2001 CBC DIVISION VII – EARTHQUAKE REGULATIONS FOR SEISMIC ISOLATED STRUCTURES

~~2001 CBC SECTION 1654A – GENERAL:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1655A – DEFINITIONS:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1656A – SYMBOLS AND NOTATIONS:~~ Repeal all amendments in this section.

2001 CBC SECTION 1657A – CRITERIA SELECTION: Repeal amendment in the following subsection.

~~1657A.5.1~~ and ~~1657A.5.3~~ Item # 1.

2001 CBC SECTION 1658A – STATIC LATERAL RESPONSE PROCEDURE: Repeal amendment in the following subsection.

~~1658A.4.3.~~

2001 CBC SECTION 1659A – DYNAMIC LATERAL RESPONSE PROCEDURE: Repeal amendment in the following subsection.

~~1659A.8.~~

2001 CBC SECTION 1660A – LATERAL LOAD ON ELEMENTS OF STRUCTURES AND NONSTRUCTURAL COMPONENTS SUPPORTED BY STRUCTURES: Repeal amendment in the following subsection.

~~1660A.1.~~

2001 CBC SECTION 1661A – DETAILED SYSTEMS REQUIREMENTS: Repeal amendment in the following subsection.

~~1661A.2.6.~~

2001 CBC SECTION 1664A – DESIGN AND CONSTRUCTION REVIEW: Repeal amendment in the following subsection.

~~1664A.3.~~

2001 CBC SECTION 1664A – DESIGN AND CONSTRUCTION REVIEW: Repeal amendment in the following subsection.

~~1664A.3.~~

2001 CBC SECTION 1665A – REDUCED TEST OF ISOLATION SYSTEM: Repeal amendment in the following subsection.

~~1665A.2.3.~~

2001 CBC CHAPTER 16A – TABLES AND FIGURES: Repeal amendment in the following table.

~~TABLE A-16-E.~~

EXPRESS TERMS

SECTION 1601A - GENERAL

1601A.1 Scope. The provisions of this chapter shall govern the structural design of buildings, structures and portions thereof regulated by this code.

1601A.1.1 Application. *The scope of application of Chapter 16A is as follows:*

- 1. Applications listed in Section 109.2, regulated by the Division of the State Architect-Structural Safety (DSA-SS). These applications include public elementary and secondary schools, community colleges and state-owned or state-leased essential services buildings.*
- 2. Applications listed in Section 110.1, and 110.4, regulated by the Office of Statewide Health Planning and Development (OSHPD). These applications include hospitals, skilled nursing facilities, intermediate care facilities and correctional treatment centers.*

Exception [For OSHPD 2]: *Single-story Type V skilled nursing or intermediate care facilities utilizing wood-frame or light-steel-frame construction as defined in Health and Safety Code Section 129725, which shall comply with CBC Chapter 16 and any applicable amendments therein.*

1601A.1.2 Amendments in this chapter. *DSA - SS and OSHPD adopt this chapter and all amendments.*

Exception: *Amendments adopted by only one agency appear in this chapter preceded with the appropriate acronym of the adopting agency, as follows:*

- 1. Division of the State Architect - Structural Safety:*

[DSA-SS] - *For applications listed in Section 109.2*

- 2. Office of Statewide Health Planning and Development:*

[OSHPD 1] - *For applications listed in Section 110.1*

[OSHPD 4] - *For applications listed in Section 110.4*

1601A.2 References. *All referenced codes and standards listed in Chapter 35 shall include all the modifications contained in this code to referenced standards. In the event of any discrepancy between this code and a referenced standard, refer to Section 101.7*

1601A.3 Enforcement Agency Approval. *In addition to requirements of California Code of Regulations (C.C.R.) Title 24, Parts 1 & 2, any aspect of project design, construction, quality assurance, or quality control programs for which this code requires approval by the design professional, are also subject to approval by the enforcement agency.*

SECTION 1602A - DEFINITIONS AND NOTATIONS

1602A.1 Definitions. The following words and terms shall, for the purposes of this chapter, have the meanings shown herein.

ALLOWABLE STRESS DESIGN. A method of proportioning structural members, such that elastically computed stresses produced in the members by nominal loads do not exceed specified allowable stresses (also called "working stress design").

BALCONY, EXTERIOR. An exterior floor projecting from and supported by a structure without additional independent supports.

DEAD LOADS. The weight of materials of construction incorporated into the building, including but not limited to walls, floors, roofs, ceilings, stairways, built-in partitions, finishes, cladding and other similarly incorporated architectural and structural items, and the weight of fixed service equipment, such as cranes, plumbing stacks and risers, electrical feeders, heating, ventilating and air-conditioning systems and fire sprinkler systems.

DECK. An exterior floor supported on at least two opposing sides by an adjacent structure, and/or posts, piers or other independent supports.

DESIGN STRENGTH. The product of the nominal strength and a resistance factor (or strength reduction factor).

DIAPHRAGM. A horizontal or sloped system acting to transmit lateral forces to the vertical-resisting elements. When the term "diaphragm" is used, it shall include horizontal bracing systems.

Diaphragm, blocked. In light-frame construction, a diaphragm in which all sheathing edges not occurring on a framing member are supported on and fastened to blocking.

Diaphragm boundary. In light-frame construction, a location where shear is transferred into or out of the diaphragm sheathing. Transfer is either to a boundary element or to another force-resisting element.

Diaphragm chord. A diaphragm boundary element perpendicular to the applied load that is assumed to take axial stresses due to the diaphragm moment.

Diaphragm flexible. A diaphragm is flexible for the purpose of distribution of story shear and torsional moment where so indicated in Section 12.3.1 of ASCE 7, as modified in Section 1613A.6.1.

Diaphragm, rigid. A diaphragm is rigid for the purpose of distribution of story shear and torsional moment when the lateral deformation of the diaphragm is less than or equal to two times the average story drift.

DURATION OF LOAD. The period of continuous application of a given load, or the aggregate of periods of intermittent applications of the same load.

(Relocated from 1641A, 2001 CBC) **ENFORCEMENT AGENT.** *That individual within the agency or organization charged with responsibility for agency or organization compliance with the requirements of this Code. Used interchangeably with Building Official and Code Official.*

ESSENTIAL FACILITIES. Buildings and other structures that are intended to remain operational in the event of extreme environmental loading from flood, wind, snow or earthquakes.

FABRIC PARTITIONS. A partition consisting of a finished surface made of fabric, without a continuous rigid backing, that is directly attached to a framing system in which the vertical framing members are spaced greater than 4 feet (1219 mm) on center.

FACTORED LOAD. The product of a nominal load and a load factor.

GUARD. See Section 1002.1.

(Relocated from 1602A, 2001 CBC) **HOSPITAL BUILDING** *for OSHPD 1, 2, 3 and 41. Any building defined in Section 129725, Health and Safety Code.*

IMPACT LOAD. The load resulting from moving machinery, elevators, craneways, vehicles and other similar forces and kinetic loads, pressure and possible surcharge from fixed or moving loads.

LIMIT STATE. A condition beyond which a structure or member becomes unfit for service and is judged to be no longer useful for its intended function (serviceability limit state) or to be unsafe (strength limit state).

LIVE LOADS. Those loads produced by the use and occupancy of the building or other structure and do not include construction or environmental loads such as wind load, snow load, rain load, earthquake load, flood load or dead load.

LIVE LOADS (ROOF). Those loads produced (1) during maintenance by workers, equipment and materials; and (2) during the life of the structure by movable objects such as planters and by people.

LOAD AND RESISTANCE FACTOR DESIGN (LRFD). A method of proportioning structural members and their connections using load and resistance factors such that no applicable limit state is reached when the structure is subjected to appropriate load combinations. The term "LRFD" is used in the design of steel and wood structures.

LOAD EFFECTS. Forces and deformations produced in structural members by the applied loads.

LOAD FACTOR. A factor that accounts for deviations of the actual load from the nominal load, for uncertainties in the analysis that transforms the load into a load effect, and for the probability that more than one extreme load will occur simultaneously.

LOADS. Forces or other actions that result from the weight of building materials, occupants and their possessions, environmental effects, differential movement and restrained dimensional changes. Permanent loads are those loads in which variations over time are rare or of small magnitude, such as dead loads. All other loads are variable loads (see also "Nominal loads").

NOMINAL LOADS. The magnitudes of the loads specified in this chapter (dead, live, soil, wind, snow, rain, flood and earthquake).

OCCUPANCY CATEGORY. A category used to determine structural requirements based on occupancy.

OTHER STRUCTURES. Structures, other than buildings, for which loads are specified in this chapter.

PANEL (PART OF A STRUCTURE). The section of a floor, wall or roof comprised between the supporting frame of two adjacent rows of columns and girders or column bands of floor or roof construction.

RESISTANCE FACTOR. A factor that accounts for deviations of the actual strength from the nominal strength and the manner and consequences of failure (also called "strength reduction factor").

STRENGTH, NOMINAL. The capacity of a structure or member to resist the effects of loads, as determined by computations using specified material strengths and dimensions and equations derived from accepted principles of structural mechanics or by field tests or laboratory tests of scaled models, allowing for modeling effects and differences between laboratory and field conditions.

STRENGTH, REQUIRED. Strength of a member, cross section or connection required to resist factored loads or related internal moments and forces in such combinations as stipulated by these provisions.

STRENGTH DESIGN. A method of proportioning structural members such that the computed forces produced in the members by factored loads do not exceed the member design strength [also called "load and resistance factor design" (LRFD)]. The term "strength design" is used in the design of concrete and masonry structural elements.

VEHICLE BARRIER SYSTEM. A system of building components near open sides of a garage floor or ramp or building walls that act as restraints for vehicles.

NOTATIONS.

D = Dead load.

E = Combined effect of horizontal and vertical earthquake induced forces as defined in Section 12.4.2 of ASCE 7.

E_m = Maximum seismic load effect of horizontal and vertical seismic forces as set forth in Section 12.4.3 of ASCE 7.

F = Load due to fluids with well-defined pressures and maximum heights.

F_a = Flood load.

H = Load due to lateral earth pressures, ground water pressure or pressure of bulk materials.

L = Live load, except roof live load, including any permitted live load reduction.

L_r = Roof live load including any permitted live load reduction.

R = Rain load.

S = Snow load.

T = Self-straining force arising from contraction or expansion resulting from temperature change, shrinkage, moisture change, creep in component materials, movement due to differential settlement or combinations thereof.

W = Load due to wind pressure.

SECTION 1603A - CONSTRUCTION DOCUMENTS

1603A.1 General. Construction documents shall show the size, section and relative locations of structural members with floor levels, column centers and offsets dimensioned. The design loads and other information pertinent to the structural design required by Sections 1603A.1.1 through 1603A.1.8 shall be indicated on the construction documents.

[For OSHPD 1] Additional requirements are included in Section 7-115 and 7-125 of the Building Standards Administration Code (Part 1, Title 24, C.C.R).

[For DSA-SS] Additional requirements are included in Section 4-115 and 4-125 of the Building Standards Administration Code (Part 1, Title 24, C.C.R).

Exception: Construction documents for buildings constructed in accordance with the conventional light-frame construction provisions of Section 2308 shall indicate the following structural design information:

1. Floor and roof live loads.
2. Ground snow load, P_g .
3. Basic wind speed (3-second gust), miles per hour (mph) (km/hr) and wind exposure.
4. Seismic design category and site class.
5. Flood design data, if located in flood hazard areas established in Section 1612A.3.

1603A.1.1 Floor live load. The uniformly distributed, concentrated and impact floor live load used in the design shall be indicated for floor areas. Use of live load reduction in accordance with Section 1607A.9 shall be indicated for each type of live load used in the design.

1603A.1.2 Roof live load. The roof live load used in the design shall be indicated for roof areas (Section 1607A.11).

1603A.1.3 Roof snow load. The ground snow load, P_g , shall be indicated. In areas where the ground snow load, P_g , exceeds 10 pounds per square foot (psf) (0.479 kN/m²), the following additional information shall also be provided, regardless of whether snow loads govern the design of the roof:

1. Flat-roof snow load, P_f .
2. Snow exposure factor, C_e .

3. Snow load importance factor, I .
4. Thermal factor, C_t .

1603A.1.4 Wind design data. The following information related to wind loads shall be shown, regardless of whether wind loads govern the design of the lateral-force-resisting system of the building:

1. Basic wind speed (3-second gust), miles per hour (km/hr).
2. Wind importance factor, I , and occupancy category.
3. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.
4. The applicable internal pressure coefficient.
5. Components and cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional.

1603A.1.5 Earthquake design data. The following information related to seismic loads shall be shown, regardless of whether seismic loads govern the design of the lateral-force-resisting system of the building:

1. Seismic importance factor, I , and occupancy category.
2. Mapped spectral response accelerations, S_S and S_I .
3. Site class.
4. Spectral response coefficients, S_{DS} and S_{DI} .
5. Seismic design category.
6. Basic seismic-force-resisting system(s).
7. Design base shear.
8. Seismic response coefficient(s), C_S .
9. Response modification factor(s), R .
10. Analysis procedure used.

1603A.1.5.1 (Relocated from 1633A.2.3, 2001 CBC) Connections. *Connections that resist design seismic forces shall be designed and detailed on the design drawings.*

1603A.1.6 Flood design data. For buildings located in whole or in part in flood hazard areas as established in Section 1612A.3, the documentation pertaining to design, if required in Section 1612A.5, shall be included and the following information, referenced to the datum on the community's Flood Insurance Rate Map (FIRM), shall be shown, regardless of whether flood loads govern the design of the building:

1. In flood hazard areas not subject to high-velocity wave action, the elevation of the proposed lowest floor, including the basement.
2. In flood hazard areas not subject to high-velocity wave action, the elevation to which any nonresidential building will be dry floodproofed.
3. In flood hazard areas subject to high-velocity wave action, the proposed elevation of the bottom of the lowest horizontal structural member of the lowest floor, including the basement.

1603A.1.7 Special loads. Special loads that are applicable to the design of the building, structure or portions thereof shall be indicated along with the specified section of this code that addresses the special loading condition.

1603A.1.8 Systems and components requiring special inspections for seismic resistance. Construction documents or specifications shall be prepared for those systems and components requiring special inspection for seismic resistance as specified in Section 1707A.1 by the registered design professional responsible for their design and shall be submitted for approval in accordance with Section 106.1, Appendix Chapter 1. Reference to seismic standards in lieu of detailed drawings is acceptable.

1603A.2 Restrictions on loading. It shall be unlawful to place, or cause or permit to be placed, on any floor or roof of a building, structure or portion thereof, a load greater than is permitted by these requirements.

1603A.3 Live loads posted. Where the live loads for which each floor or portion thereof of a commercial or industrial building is or has been designed to exceed 50 psf (2.40 kN/m²), such design live loads shall be conspicuously posted by the owner in that part of each story in which they apply, using durable signs. It shall be unlawful to remove or deface such notices.

1603A.3.1 (Relocated from 1614A.1, 2001 CBC) Snow Load Posting. *Snow loads used in design shall be posted as for live loads. See Section 1607A.3.5. Snow accumulation removal shall begin when the depth of snow creates loadings of 75 percent of the design values.*

1603A.3.2 (Relocated from 1607A.3.5.2, 2001 CBC) [For OSHPD 1 & 4] Load Posting Responsibility. *The hospital owner or hospital governing board shall be responsible for keeping the actual load below the allowable limits.*

1603A.4 Occupancy permits for changed loads. Occupancy permits for buildings hereafter erected shall not be issued until the floor load signs, required by Section 1603A.3, have been installed.

SECTION 1604A GENERAL DESIGN REQUIREMENTS

1604A.1 General. Building, structures and parts thereof shall be designed and constructed in accordance with strength design, load and resistance factor design, allowable stress design, empirical design or conventional construction methods, as permitted by the applicable material chapters.

1604A.2 Strength. Buildings and other structures, and parts thereof, shall be designed and constructed to support safely the factored loads in load combinations defined in this code without exceeding the appropriate strength limit states for the materials of construction. Alternatively, buildings and other structures, and parts thereof, shall be designed and constructed to support safely the nominal loads in load combinations defined in this code without exceeding the appropriate specified allowable stresses for the materials of construction.

Loads and forces for occupancies or uses not covered in this chapter shall be subject to the approval of the building official.

1604A.3 Serviceability. Structural systems and members thereof shall be designed to have adequate stiffness to limit deflections and lateral drift. See Section 12.12.1 of ASCE 7 for drift limits applicable to earthquake loading.

TABLE 1604A.3 - DEFLECTION LIMITS^{a, b, c, h, i}

CONSTRUCTION	L	S or W	D + L ^{d,g}
Roof members: ^c			
Supporting plaster ceiling	l/360	l/360	l/240
Supporting nonplaster ceiling	l/240	l/240	l/180
Not supporting ceiling	l/180	l/180	l/120
Floor members	l/360	—	l/240
Exterior walls and interior partitions:			
With brittle finishes	—	l/240	—
With flexible finishes	—	l/120 l/180	—

(Relocated from Table 16A-W, 2001 CBC)

<u>Veneered walls, anchored veneers and adhered veneers over 1 inch (25 mm) thick, including the mortar backing</u>		<u>l/480</u>	
Farm buildings	—	—	<u>l/180</u>
Greenhouses	—	—	<u>l/120</u>

For SI: 1 foot = 304.8 mm.

- a. For structural roofing and siding made of formed metal sheets, the total load deflection shall not exceed $l/60$. For secondary roof structural members supporting formed metal roofing, the live load deflection shall not exceed $l/150$. For secondary wall members supporting formed metal siding, the design wind load deflection shall not exceed $l/90$. For roofs, this exception only applies when the metal sheets have no roof covering.
- b. Interior partitions not exceeding 6 feet in height and flexible, folding and portable partitions are not governed by the provisions of this section. The deflection criterion for interior partitions is based on the horizontal load defined in Section 1607A.13.
- c. See Section 2403 for glass supports.
- d. For wood structural members having a moisture content of less than 16 percent at time of installation and used under dry conditions, the deflection resulting from $L + 0.5D$ is permitted to be substituted for the deflection resulting from $L + D$.
- e. The above deflections do not ensure against ponding. Roofs that do not have sufficient slope or camber to assure adequate drainage shall be investigated for ponding. See Section 1611A for rain and ponding requirements and Section 1503.4 for roof drainage requirements.
- f. The wind load is permitted to be taken as 0.7 times the "component and cladding" loads for the purpose of determining deflection limits herein.
- g. For steel structural members, the dead load shall be taken as zero.
- h. For aluminum structural members or aluminum panels used in skylights and sloped glazing framing, roofs or walls of sunroom additions or patio covers, not supporting edge of glass or aluminum sandwich panels, the total load deflection shall not exceed $l/60$. For aluminum sandwich panels used in roofs or walls of sunroom additions or patio covers, the total load deflection shall not exceed $l/120$.
- i. For cantilever members, l shall be taken as twice the length of the cantilever.

1604A.3.1 Deflections. The deflections of structural members shall not exceed the more restrictive of the limitations of Sections 1604A.3.2 through ~~1604.3.5~~ **1604A.3.8** or that permitted by Table 1604A.3.

1604A.3.2 Reinforced concrete. The deflection of reinforced concrete structural members shall not exceed that permitted by ACI 318.

1604A.3.3 Steel. The deflection of steel structural members shall not exceed that permitted by AISC 360, AISI-NAS, AISI-General, AISI-Truss, ASCE 3, ASCE 8, SJI JG-1.1, SJI K-1.1 or SJI LH/DLH-1.1, as applicable.

1604A.3.4 Masonry. The deflection of masonry structural members shall not exceed that permitted by ACI 530/ASCE 5/TMS 402.

1604A.3.5 Aluminum. The deflection of aluminum structural members shall not exceed that permitted by AA ADM1.

1604A.3.6 Limits. Deflection of structural members over span, l , shall not exceed that permitted by Table 1604A.3.

1604A.3.7 (Relocated from 1613A.2, 2001 CBC) Lateral Load Deflections.

1604A.3.7.1 (Relocated from 1613A.2.1, 2001 CBC) General. The deflection of structural systems designed to resist wind or seismic loads shall be such that other portions of the structure are not overstressed.

NOTE: See ~~Section 1633A.2.4~~ ASCE 7 Section 12.12.4.

1604A.3.7.2 (Relocated from 1613A.2.3, 2001 CBC) **Horizontal diaphragms.** The maximum span-width ratio for any roof or floor diaphragm shall not exceed those given in Table 16A-V 2305.2.3 or ICC-ES AC 43 unless test data and design calculations acceptable to the enforcement agency are submitted and approved for the use of other span-width ratios. Concrete diaphragm shall not exceed span-width ratios for equivalent composite floor diaphragm in ICC-ES AC 43.

1604A.3.8 (Relocated from 1613A.1, 2001 CBC) **Deflections.** Deflection criteria for materials not specified shall be developed by the project architect or structural engineer in a manner consistent with the provisions of this section and approved by the enforcement agency.

1604A.4 Analysis. Load effects on structural members and their connections shall be determined by methods of structural analysis that take into account equilibrium, general stability, geometric compatibility and both short- and long-term material properties.

Members that tend to accumulate residual deformations under repeated service loads shall have included in their analysis the added eccentricities expected to occur during their service life.

Any system or method of construction to be used shall be based on a rational analysis in accordance with well-established principles of mechanics. Such analysis shall result in a system that provides a complete load path capable of transferring loads from their point of origin to the load-resisting elements.

The total lateral force shall be distributed to the various vertical elements of the lateral-force-resisting system in proportion to their rigidities, considering the rigidity of the horizontal bracing system or diaphragm. Rigid elements assumed not to be a part of the lateral-force-resisting system are permitted to be incorporated into buildings provided their effect on the action of the system is considered and provided for in the design. Except where diaphragms are flexible, or are permitted to be analyzed as flexible, provisions shall be made for the increased forces induced on resisting elements of the structural system resulting from torsion due to eccentricity between the center of application of the lateral forces and the center of rigidity of the lateral-force-resisting system.

Every structure shall be designed to resist the overturning effects caused by the lateral forces specified in this chapter. See Section 1609A for wind loads, Section 1610A for lateral soil loads and Section 1613A for earthquake loads.

1604A.5 Occupancy category. Buildings shall be assigned an occupancy category in accordance with Table 1604A.5.

TABLE 1604A.5 - OCCUPANCY CATEGORY OF BUILDINGS AND OTHER STRUCTURES

OCCUPANCY CATEGORY	NATURE OF OCCUPANCY
I	Buildings and other structures that represent a low hazard to human life in the event of failure, including but not limited to: <ul style="list-style-type: none"> • Agricultural facilities. • Certain temporary facilities. • Minor storage facilities.
II	Buildings and other structures except those listed in Occupancy Categories I, III and IV
III	Buildings and other structures that represent a substantial hazard to human life in the event of failure, including but not limited to: <ul style="list-style-type: none"> • Covered structures whose primary occupancy is public assembly with an occupant load greater than 300. • Buildings and other structures with elementary school, secondary school or day care facilities with an occupant load greater than 250. • Buildings and other structures with an occupant load greater than 500 for colleges or adult education facilities.

	<ul style="list-style-type: none"> • <u>[For OSHPD 1 & 4] Not permitted by OSHPD. Health care facilities with an occupant load of 50 or more resident patients, but not having surgery or emergency treatment facilities.</u> • Jails and detention facilities. • Any other occupancy with an occupant load greater than 5,000. • Power-generating stations, water treatment for potable water, waste water treatment facilities and other public utility facilities not included in Occupancy Category IV. • Buildings and other structures not included in Occupancy Category IV containing sufficient quantities of toxic or explosive substances to be dangerous to the public if released.
IV	<p>Buildings and other structures designated as essential facilities, including but not limited to:</p> <ul style="list-style-type: none"> • <u>[For OSHPD 1 & 4] (Relocated from Table 16A-K, 2001 CBC) Hospitals and other health care facilities as defined in Section 1250, Health and Safety Code. having surgery or emergency treatment facilities. <u>Hospital Buildings as defined in C.C.R. Title 24, Part 1, Section 7-111 and all structures required for their continuous operation and access.</u></u> • Fire, rescue and police stations and emergency vehicle garages. • Designated earthquake, hurricane or other emergency shelters. • Designated emergency preparedness, communication, and operation centers and other facilities required for emergency response <u>[For DSA-SS] as defined in C.C.R. Title 24, Part 1, Section 4-111 and all structures required for their continuous operation and access.</u> • Power-generating stations and other public utility facilities required as emergency backup facilities for Occupancy Category IV structures. • Structures containing highly toxic materials as defined by Section 307 where the quantity of the material exceeds the maximum allowable quantities of Table 307.1.(2). • Aviation control towers, air traffic control centers and emergency aircraft hangars. • Buildings and other structures having critical national defense functions. • Water treatment facilities required to maintain water pressure for fire suppression.

1604A.5.1 Multiple occupancies. Where a structure is occupied by two or more occupancies not included in the same occupancy category, the structure shall be assigned the classification of the highest occupancy category corresponding to the various occupancies. Where structures have two or more portions that are structurally separated, each portion shall be separately classified. Where a separated portion of a structure provides required access to, required egress from or shares life safety components with another portion having a higher occupancy category, both portions shall be assigned to the higher occupancy category.

1604A.6 In-situ load tests. The building official is authorized to require an engineering analysis or a load test, or both, of any construction whenever there is reason to question the safety of the construction for the intended occupancy. Engineering analysis and load tests shall be conducted in accordance with Section 1713A.

1604A.7 Preconstruction load tests. Materials and methods of construction that are not capable of being designed by approved engineering analysis or that do not comply with the applicable material design standards listed in Chapter 35, or alternative test procedures in accordance with Section 1711A, shall be load tested in accordance with Section 1714A.

1604A.8 Anchorage.

1604A.8.1 General. Anchorage of the roof to walls and columns, and of walls and columns to foundations, shall be provided to resist the uplift and sliding forces that result from the application of the prescribed loads.

1604A.8.2 Concrete and masonry walls. Concrete and masonry walls shall be anchored to floors, roofs and other structural elements that provide lateral support for the wall. Such anchorage shall provide a positive direct connection capable of resisting the horizontal forces specified in this chapter but not less than a minimum strength design horizontal force of 280 plf (4.10 kN/m) of wall, substituted for "E" in the load combinations of Section 1605A.2 or 1605A.3. Walls shall be designed to resist bending between anchors where the anchor spacing exceeds 4 feet (1219 mm). Required anchors in masonry walls of hollow units or cavity walls shall be embedded in a reinforced grouted structural element of the wall. See Sections 1609A for wind design requirements and see Section 1613A for earthquake design requirements.

1604A.8.3 Decks. Where supported by attachment to an exterior wall, decks shall be positively anchored to the primary structure and designed for both vertical and lateral loads as applicable. Such attachment shall not be accomplished by the use of toenails or nails subject to withdrawal. Where positive connection to the primary building structure cannot be verified during inspection, decks shall be self-supporting. For decks with cantilevered framing members, connections to exterior walls or other framing members shall be designed and constructed to resist uplift resulting from the full live load specified in Table 1607A.1 acting on the cantilevered portion of the deck.

1604A.9 Counteracting structural actions. Structural members, systems, components and cladding shall be designed to resist forces due to earthquake and wind, with consideration of overturning, sliding, and uplift. Continuous load paths shall be provided for transmitting these forces to the foundation. Where sliding is used to isolate the elements, the effects of friction between sliding elements shall be included as a force.

1604A.10 Wind and seismic detailing. Lateral-force-resisting systems shall meet seismic detailing requirements and limitations prescribed in this code and ASCE 7, excluding Chapter 14 and Appendix 11A, even when wind code prescribed load effects are greater than seismic load effects.

1604A.11 (Relocated from 1605A.5, 2001 CBC) Construction Procedures. *Where unusual erection or construction procedures are considered essential by the project-structural engineer or architect in order to accomplish the intent of the design or influence the design, such procedure shall be indicated on the plans or in the specifications and shall have the prior approval of the enforcement agency.*

SECTION 1605A - LOAD COMBINATIONS

1605A.1 General. Buildings and other structures and portions thereof shall be designed to resist the load combinations specified in Section 1605A.2 or 1605A.3 and Chapters 18A through 23, and the special seismic load combinations of Section 1605A.4 where required by Section 12.3.3.3 or 12.10.2.1 of ASCE 7. Applicable loads shall be considered, including both earthquake and wind, in accordance with the specified load combinations. Each load combination shall also be investigated with one or more of the variable loads set to zero.

1605A.2 Load combinations using strength design or load and resistance factor design.

1605A.2.1 Basic load combinations. Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:

$$1.4 (D + F)$$

(Equation 16A-1)

$$1.2(D + F + T) + 1.6(L + H) + 0.5 (L_r \text{ or } S \text{ or } R)$$

(Equation 16A-2)

$$1.2D + 1.6(L_r \text{ or } S \text{ or } R) + (f_1 L \text{ or } 0.8W)$$

(Equation 16A-3)

$$1.2D + 1.6W + f_1 L + 0.5(L_r \text{ or } S \text{ or } R)$$

(Equation 16A-4)

$$1.2D + 1.0E + f_1 L + f_2 S$$

(Equation 16A-5)

$$0.9D + 1.6W + 1.6H$$

(Equation 16A-6)

$$0.9D + 1.0E + 1.6H$$

(Equation 16A-7)

$f_1 = 1$ for floors in places of public assembly, for live loads in excess of 100 pounds per square foot (4.79 kN/m²), and for parking garage live load, and

= 0.5 for other live loads.

$f_2 = 0.7$ for roof configurations (such as saw tooth) that do not shed snow off the structure, and

= 0.2 for other roof configurations.

Exception: Where other factored load combinations are specifically required by the provisions of this code, such combinations shall take precedence.

1605A.2.1.1 [For DSA-SS] Determination of f_2 . *The value of f_2 shall conform with the requirements adopted by the city, county, or city and county in which the project is located, if more restrictive than prescribed in Section 1605A.2.1.*

1605A.2.2 Other loads. Where F_d is to be considered in the design, the load combinations of Section 2.3.3 of ASCE 7 shall be used.

1605A.3 Load combinations using allowable stress design.

1605A.3.1 Basic load combinations. Where allowable stress design (working stress design), as permitted by this code, is used, structures and portions thereof shall resist the most critical effects resulting from the following combinations of loads:

$$D + F$$

(Equation 16A-8)

$$D + H + F + L + T$$

(Equation 16A-9)

$$D + H + F + (L_r \text{ or } S \text{ or } R)$$

(Equation 16A-10)

$$D + H + F + 0.75(L + T) + 0.75 (L_r \text{ or } S \text{ or } R)$$

(Equation 16A-11)

$$D + H + F + (W \text{ or } 0.7E)$$

(Equation 16A-12)

$$D + H + F + 0.75(W \text{ or } 0.7E) + 0.75L + 0.75 (L_r \text{ or } S \text{ or } R)$$

(Equation 16A-13)

$$0.6D + W + H$$

(Equation 16A-14)

$$0.6D + 0.7E + H$$

(Equation 16A-15)

Exceptions:

1. Crane hook loads need not be combined with roof live load or with more than three-fourths of the snow load or one-half of the wind load.
2. Flat roof snow loads of 30 psf (1.44 kN/m²) or less need not be combined with seismic loads. Where flat roof snow loads exceed 30 psf (1.44 kN/m²), 20 percent shall be combined with seismic loads.

1605A.3.1.1 Stress increases. Increases in allowable stresses specified in the appropriate material chapter or the referenced standards shall not be used with the load combinations of Section 1605A.3.1, except that a duration of load increase shall be permitted in accordance with Chapter 23.

1605A.3.1.2 Other loads. Where F_a is to be considered in design, the load combinations of Section 2.4.2 of ASCE 7 shall be used.

1605A.3.2 Alternative basic load combinations. In lieu of the basic load combinations specified in Section 1605A.3.1, structures and portions thereof shall be permitted to be designed for the most critical effects resulting from the following combinations. When using these alternative basic load combinations that include wind or seismic loads, allowable stresses are permitted to be increased or load combinations reduced where permitted by the material chapter of this code or the referenced standards.

(Relocated from 1632A.1, 2001 CBC) Intermittent connections such as inserts for anchorage of nonstructural components shall not be allowed the one-third increase in allowable stresses.

For load combinations that include the counteracting effects of dead and wind loads, only two-thirds of the minimum dead load likely to be in place during a design wind event shall be used. Where wind loads are calculated in accordance with Chapter 6 of ASCE 7, the coefficient ω in the following equations shall be taken as 1.3. For other wind loads, ω shall be taken as 1. When using these alternative load combinations to evaluate sliding, overturning and soil bearing at the soil-structure interface, the reduction of foundation overturning from Section 12.13.4 in ASCE 7 shall not be used. When using these alternative basic load combinations for proportioning foundations for loadings, which include seismic loads, the vertical seismic load effect, E_v , in Equation 12.4-4 of ASCE 7 is permitted to be taken equal to zero.

$D + L + (L_r \text{ or } S \text{ or } R)$
(Equation 16A-16)

$D + L + (\omega W)$
(Equation 16A-17)

$D + L + \omega W + S/2$
(Equation 16A-18)

$D + L + S + \omega W/2$
(Equation 1A-19)

$D + L + S + E/1.4$
(Equation 16A-20)

$0.9D + E/1.4$
(Equation 16A-21)

Exceptions:

1. Crane hook loads need not be combined with roof live loads or with more than three-fourths of the snow load or one-half of the wind load.
2. Flat roof snow loads of 30 psf (1.44 kN/m²) or less need not be combined with seismic loads. Where flat roof snow loads exceed 30 psf (1.44 kN/m²), 20 percent shall be combined with seismic loads.

1605A.3.2.1 Other loads. Where F , H or T are to be considered in the design, each applicable load shall be added to the combinations specified in Section 1605A.3.2.

1605A.4 Special seismic load combinations. For both allowable stress design and strength design methods where specifically required by Section 1605A.1 or by Chapters 18A through 23, elements and components shall be designed to resist the forces calculated using Equation 16A-22 when the effects of the seismic ground motion are additive to gravity forces and those calculated using Equation 16A-23 when the effects of the seismic ground motion counteract gravity forces.

$1.2D + f_1L + E_m$
(Equation 16A-22)

$0.9D + E_m$
(Equation 16A-23)

where:

E_m = The maximum effect of horizontal and vertical forces as set forth in Section 12.4.3 of ASCE 7.

$f_1 = 1$ for floors in places of public assembly, for live loads in excess of 100 psf (4.79 kN/m²) and for parking garage live load, or

= 0.5 for other live loads.

1605A.5 Heliports and helistops. Heliport and helistop landing areas shall be designed for the following loads, combined in accordance with Section 1605A:

1. Dead load, D , plus the gross weight of the helicopter, D_h , plus snow load, S .
2. Dead load, D , plus two single concentrated impact loads, L , approximately 8 feet (2438 mm) apart applied anywhere on the landing area (representing the helicopter's two main landing gear, whether skid type or wheeled type), having a magnitude of 0.75 times the gross weight of the helicopter. Both loads acting together total one-and one half times the gross weight of the helicopter.
3. Dead load, D , plus a uniform live load, L , of 100 psf (4.79 kN/m²).

Exception: ~~Not permitted by OSHPD and DSA-SS. Landing areas designed for helicopters with gross weights not exceeding 3,000 pounds (13.34 kN) in accordance with Items 1 and 2 shall be permitted to be designed using a 40 psf (1.92 kN/m²) uniform live load in Item 3, provided the landing area is identified with a 3,000 pound (13.34 kN) weight limitation. This 40 psf (1.92 kN/m²) uniform live load shall not be reduced. The landing area weight limitation shall be indicated by the numeral "3" (kips) located in the bottom right corner of the landing area as viewed from the primary approach path. The landing area weight limitation shall be a minimum of 5 feet (1524 mm) in height.~~

SECTION 1606A - DEAD LOADS

1606A.1 General. Dead loads are those loads defined in Section 1602A.1. Dead loads shall be considered permanent loads.

1606A.2 Design dead load. For purposes of design, the actual weights of materials of construction and fixed service equipment shall be used. In the absence of definite information, values used shall be subject to the approval of the building official.

1606A.3 (Relocated from 1607A.4.1, 2001 CBC) Roof Dead Loads. ~~The design dead load shall provide for the weight of at least one reroofing additional roof covering in addition to other applicable loadings if the new reroofing roof covering can be applied over the original roofing without its removal, in accordance with Section 1510.~~

SECTION 1607A - LIVE LOADS

1607A.1 General. Live loads are those loads defined in Section 1602A.1.

TABLE 1607A.1 - MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS AND MINIMUM CONCENTRATED LIVE LOADS⁹

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
1. Apartments (see residential)	—	—
2. Access floor systems		

Office use	50	2,000
Computer use	100	2,000
3. Armories and drill rooms	150	—
4. Assembly areas and theaters <i>n,p</i>		
Fixed seats (fastened to floor)	60	—
Follow spot, projections and control rooms	50	
Lobbies	100	
Movable seats	125	
Stages and platforms	100	
5. Balconies	60	—
On one- and two-family residences only, and not exceeding 100 sq ft		
6. Bowling Alleys	75	—
7. Catwalks	40	300
8. Dance Halls and ballrooms	100	—
9. Decks	Same as occupancy served ^h	—
10. Dining rooms and restaurants	100	—
11. Dwellings (see residential)	—	—
12. Cornices	60	—
13. Corridors, except as otherwise indicated	100	—
14. Elevator machine room grating (on area of 4 in2)	—	300
15. Finish light floor plate construction (on area of 1 in2)	—	200
16. Fire escapes	100	—
On single-family dwellings only	40	
17. Garages (passenger vehicles only)	40	Note a
Trucks and buses		See Section 1607A.6
18. Grandstands (see stadium and arena bleachers)	—	—
19. Gymnasiums <i>p</i> , main floors and balconies	100	—
20. Handrails, guards and grab bars		See Section 1607A.7
21. Hospitals <i>(Relocated from Table 16A-A, 2001 CBC) [OSHPD 1 & 4]</i>		
Corridors above first floor	80 <u>100</u>	1,000
Operating rooms, laboratories	60	1,000
Patient rooms	40	1,000
<i>Mechanical and electrical equipment areas including open areas around equipment</i>	<u>50</u>	—
<i>Storage:</i>		
<i>Light</i>	<u>125</u>	—
<i>Heavy</i>	<u>250</u>	—
<i>Dinning Area (Not used for assembly)</i>	<u>100</u>	<u>1000</u>
<i>Kitchen and serving areas</i>	<u>50</u>	<u>1000</u>

22. Hotels (see residential)	—	—
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OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
23. Libraries ^m		
Corridors above first floor	80	1,000
Reading rooms	60	1,000
Stack rooms	150 ^b	1,000
24. Manufacturing		
Heavy	250	3,000
Light	125	2,000
25. Marquees	75	—
26. Office buildings ^m		
Corridors above first floor	80	2,000
File and computer rooms shall be designed for heavier loads based on anticipated occupancy	—	—
Lobbies and first-floor corridors	50	2,000
Offices	100	2,000
27. Penal institutions		
Cell blocks	40	—
Corridors	100	—
28. Residential		
One- and two-family dwellings		
Uninhabitable attics without storage ⁱ	10	
Uninhabitable attics with limited storage ^{i,j,k}	20	
Habitable attics and sleeping areas	30	
All other areas except balconies and decks	40	—
Hotels and multiple-family dwellings		
Private rooms and corridors serving them	40	
Public rooms and corridors serving them	100	
29. Reviewing stands, grandstands and bleachers ^p	Note c	
30. Roofs		300
All roof surfaces subject to maintenance workers		
Awnings and canopies		
Fabric construction supported by a lightweight rigid skeleton structure	5 nonreduceable	
All other construction	20	
Ordinary flat, pitched, and curved roofs	20	
Primary roof members, exposed to a work floor		
Single panel point of lower chord of roof trusses or any point along primary structural members supporting roofs:		
Over manufacturing, storage		

warehouses, and repair garages		2,000
All other occupancies		300
Roofs used for other special purposes	Note 1	Note 1
Roofs used for promenade purposes	60	
Roofs used for roof gardens or assembly purposes	100	

(continued)

TABLE 1607A.1—continued MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS AND MINIMUM CONCENTRATED LIVE LOADS⁹

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
31. Schools ^m		
Classrooms	40 ^o	1,000
Corridors above first floor	80	1,000
First-floor corridors	100	1,000
32. Scuttles, skylight ribs and accessible ceilings	—	200
33. Sidewalks, vehicular driveways and yards, subject to trucking	250 ^d	8,000 ^e
34. Skating rinks	100	—
35. Stadiums and arenas		
Bleachers ^p	100 ^c	—
Fixed seats (fastened to floor)	60 ^c	
36. Stairs and exits		Note f
One- and two-family dwellings	40	
All other	100	
37. Storage warehouses (shall be designed for heavier loads if required for anticipated storage)		
Heavy	250	
Light	125	
38. Stores		
Retail		
First floor	100	1,000
Upper floors	75	1,000
Wholesale, all floors	125	1,000
39. Vehicle barriers	See Section 1607A.7.7.3	
40. Walkways and elevated platforms (other than exitways)	60	—
41. Yards and terraces, pedestrians ^q	100	—
42. (Relocated from Table 16A-B, 2001 CBC) Storage racks and wall-hung cabinets.	Total Loads^m	

For SI: 1 inch = 25.4 mm, 1 square inch = 645.16 mm²
1 square foot = 0.0929 m²,
1 pound per square foot = 0.0479 kN/m², 1 pound = 0.004448 kN,
1 pound per cubic foot = 16 kg/m³

- a. Floors in garages or portions of buildings used for the storage of motor vehicles shall be designed for the uniformly distributed live loads of Table 1607A.1 or the following concentrated loads: (1) for garages restricted to vehicles accommodating not more than nine passengers, 3,000 pounds acting on an area of 4.5 inches by 4.5 inches; (2) for

mechanical parking structures without slab or deck which are used for storing passenger vehicles only, 2,250 pounds per wheel.

- b. The loading applies to stack room floors that support nonmobile, double-faced library bookstacks, subject to the following limitations:
 - 1. The nominal bookstack unit height shall not exceed 90 inches;
 - 2. The nominal shelf depth shall not exceed 12 inches for each face; and
 - 3. Parallel rows of double-faced bookstacks shall be separated by aisles not less than 36 inches wide.
- c. Design in accordance with the ICC *Standard on Bleachers, Folding and Telescopic Seating and Grandstands*.
- d. Other uniform loads in accordance with an approved method which contains provisions for truck loadings shall also be considered where appropriate.
- e. The concentrated wheel load shall be applied on an area of 20 square inches.
- f. Minimum concentrated load on stair treads (on area of 4 square inches) is 300 pounds.
- g. Where snow loads occur that are in excess of the design conditions, the structure shall be designed to support the loads due to the increased loads caused by drift buildup or a greater snow design determined by the building official (see Section 1608A). For special-purpose roofs, see Section 1607A.11.2.2.
- h. See Section 1604A.8.3 for decks attached to exterior walls.
- i. Attics without storage are those where the maximum clear height between the joist and rafter is less than 42 inches, or where there are not two or more adjacent trusses with the same web configuration capable of containing a rectangle 42 inches high by 2 feet wide, or greater, located within the plane of the truss. For attics without storage, this live load need not be assumed to act concurrently with any other live load requirements.
- j. For attics with limited storage and constructed with trusses, this live load need only be applied to those portions of the bottom chord where there are two or more adjacent trusses with the same web configuration capable of containing a rectangle 42 inches high by 2 feet wide or greater, located within the plane of the truss. The rectangle shall fit between the top of the bottom chord and the bottom of any other truss member, provided that each of the following criteria is met:
 - i. The attic area is accessible by a pull-down stairway or framed opening in accordance with Section 1209.2, and
 - ii. The truss shall have a bottom chord pitch less than 2:12.
 - iii. Bottom chords of trusses shall be designed for the greater of actual imposed dead load or 10 psf, uniformly distributed over the entire span.
- k. Attic spaces served by a fixed stair shall be designed to support the minimum live load specified for habitable attics and sleeping rooms.
- l. Roofs used for other special purposes shall be designed for appropriate loads as approved by the building official.

m. *(Relocated from Table 16A-B, 2001 CBC) The minimum vertical design live load shall be as follows:*

Paper media:

12-inch-deep (305 mm) shelf 33 pounds per lineal foot (482 N/m)
15-inch-deep (381 mm) shelf 41 pounds per lineal foot (598 N/m), or
33 pounds per cubic foot (5183 N/m³) per total volume of the rack or cabinet, whichever is less.

Film media:

18-inch-deep (457 mm) shelf 100 pounds per lineal foot (1459 N/m), or
50 pounds per cubic foot (7853 N/m³) per total volume of the rack or cabinet, whichever is less.

Other media:

20 pounds per cubic foot (311 N/m³) or 20 pounds per square foot (958 Pa), whichever is less, but not less than actual loads.

n. *(Relocated from Table 16A-B, 2001 CBC) [For DSA-SS] The following minimum loads for stage*

accessories apply:

1. Gridirons and fly galleries: 75 pounds per square foot uniform live load.
 2. Loft block wells: 250 pounds per lineal foot vertical load and lateral load.
 3. Head block wells and sheave beams: 250 pounds per lineal foot vertical load and lateral load. All loads are in pounds per lineal foot. Head block wells and sheave beams shall be designed for all tributary loft block well loads. Sheave blocks shall be designed with a safety factor of five.
 4. Scenery beams where there is no gridiron: 300 pounds per lineal foot vertical load and lateral load.
 5. Ceiling framing over stages shall be designed for a uniform live load of 20 pounds per square foot. Ceiling framing or roof framing above stages shall be designed for a uniform load of 20 pounds per square foot (0.96 kN/m²) in addition to other dead and live loads. For members supporting a tributary area of 200 square feet or more, this additional load may be reduced to 15 pounds per square foot (0.72 kN/m²).
- o. (Relocated from Table 16A-A, 2001 CBC) [For DSA-SS] The minimum uniform live load for classroom occupancies is 50 psf.*
- p. (Relocated from Table 16A-A, 2001 CBC) [For DSA-SS] The minimum uniform live load for a press box floor or accessible roof with railing is 100 psf.*
- q. (Relocated from Table 16A-A, 2001 CBC) [For DSA-SS] Item 41 applies to pedestrian bridges and walkways that are not subjected to uncontrolled vehicle access.*

1607A.2 Loads not specified. For occupancies or uses not designated in Table 1607A.1, the live load shall be determined in accordance with a method approved by the building official.

1607A.3 Uniform live loads. The live loads used in the design of buildings and other structures shall be the maximum loads expected by the intended use or occupancy but shall in no case be less than the minimum uniformly distributed unit loads required by Table 1607A.1.

1607A.4 Concentrated loads. Floors and other similar surfaces shall be designed to support the uniformly distributed live loads prescribed in Section 1607A.3 or the concentrated load, in pounds (kilonewtons), given in Table 1607A.1, whichever produces the greater load effects. Unless otherwise specified, the indicated concentration shall be assumed to be uniformly distributed over an area 2.5 feet by 2.5 feet [6.25 square feet (0.58 m²)] and shall be located so as to produce the maximum load effects in the structural members.

1607A.5 Partition loads. In office buildings and in other buildings where partition locations are subject to change, provisions for partition weight shall be made, whether or not partitions are shown on the construction documents, unless the specified live load exceeds 80 psf (3.83 kN/m²). The partition load shall not be less than a uniformly distributed live load of 15 psf (0.74 kN/m²).

1607A.6 Truck and bus garages. Minimum live loads for garages having trucks or buses shall be as specified in Table 1607A.6, but shall not be less than 50 psf (2.40 kN/m²), unless other loads are specifically justified and approved by the building official. Actual loads shall be used where they are greater than the loads specified in the table.

TABLE 1607A.6 - UNIFORM AND CONCENTRATED LOADS

LOADING CLASS ^a	UNIFORM LOAD (pounds/linear foot of lane)	CONCENTRATED LOAD (pounds) ^b	
		For moment design	For shear design
H20-44 and HS20-44	640	18,000	26,000
H15-44 and	480	13,500	19,500

HS15-44			
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For SI: 1 pound per linear foot = 0.01459 kN/m, 1 pound = 0.004448 kN,
1 ton = 8.90 kN.

- a. An H loading class designates a two-axle truck with a semitrailer. An HS loading class designates a tractor truck with a semitrailer. The numbers following the letter classification indicate the gross weight in tons of the standard truck and the year the loadings were instituted.
- b. See Section 1607A.6.1 for the loading of multiple spans.

1607A.6.1 Truck and bus garage live load application. The concentrated load and uniform load shall be uniformly distributed over a 10-foot (3048 mm) width on a line normal to the centerline of the lane placed within a 12-foot-wide (3658 mm) lane. The loads shall be placed within their individual lanes so as to produce the maximum stress in each structural member. Single spans shall be designed for the uniform load in Table 1607A.6 and one simultaneous concentrated load positioned to produce the maximum effect. Multiple spans shall be designed for the uniform load in Table 1607A.6 on the spans and two simultaneous concentrated loads in two spans positioned to produce the maximum negative moment effect. Multiple span design loads, for other effects, shall be the same as for single spans.

1607A.7 Loads on handrails, guards, grab bars and vehicle barriers. Handrails, guards, grab bars as designed in ICC A117.1 and vehicle barriers shall be designed and constructed to the structural loading conditions set forth in this section.

1607A.7.1 Handrails and guards. Handrail assemblies and guards shall be designed to resist a load of 50 plf (0.73 kN/m) applied in any direction at the top and to transfer this load through the supports to the structure. Glass handrail assemblies and guards shall also comply with Section 2407.

Exceptions:

1. For one- and two-family dwellings, only the single concentrated load required by Section 1607A.7.1.1 shall be applied.
2. In Group I-3, F, H and S occupancies, for areas that are not accessible to the general public and that have an occupant load less than 50, the minimum load shall be 20 pounds per foot (0.29 kN/m).

1607A.7.1.1 Concentrated load. Handrail assemblies and guards shall be able to resist a single concentrated load of 200 pounds (0.89 kN), applied in any direction at any point along the top, and have attachment devices and supporting structure to transfer this loading to appropriate structural elements of the building. This load need not be assumed to act concurrently with the loads specified in the preceding paragraph.

1607A.7.1.2 Components. Intermediate rails (all those except the handrail), balusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds (0.22 kN) on an area equal to 1 square foot (0.093m²), including openings and space between rails. Reactions due to this loading are not required to be superimposed with those of Section 1607A.7.1 or 1607A.7.1.1.

1607A.7.1.3 Stress increase. Where handrails and guards are designed in accordance with the provisions for allowable stress design (working stress design) exclusively for the loads specified in Section 1607A.7.1, the allowable stress for the members and their attachments are permitted to be increased by one-third.

1607A.7.2 Grab bars, shower seats and dressing room bench seats. Grab bars, shower seats and dressing room bench seat systems shall be designed to resist a single concentrated load of 250 pounds (1.11 kN) applied in any direction at any point.

1607A.7.3 Vehicle barriers. Vehicle barrier systems for passenger cars shall be designed to resist a single load of 6,000 pounds (26.70 kN) applied horizontally in any direction to the barrier system and shall have anchorage or attachment capable of transmitting this load to the structure. For design of the system, the load shall be assumed to act at a minimum height of 1 foot, 6 inches (457 mm) above the floor or ramp surface on an area not to exceed 1 square foot (305 mm²), and is not required to be assumed to act concurrently with any handrail or guard loadings specified in the preceding paragraphs of Section 1607A.7.1. Garages accommodating trucks and buses shall be designed in accordance with an approved method that contains provision for traffic railings.

1607A.8 Impact loads. The live loads specified in Section 1607A.3 include allowance for impact conditions. Provisions shall be made in the structural design for uses and loads that involve unusual vibration and impact forces.

1607A.8.1 Elevators. Elevator loads shall be increased by 100 percent for impact and the structural supports shall be designed within the limits of deflection prescribed by ASME A17.1.

1607A.8.2 Machinery. For the purpose of design, the weight of machinery and moving loads shall be increased as follows to allow for impact: (1) elevator machinery, 100 percent; (2) light machinery, shaft- or motor-driven, 20 percent; (3) reciprocating machinery or power-driven units, 50 percent; (4) hangers for floors or balconies, 33 percent. Percentages shall be increased where specified by the manufacturer.

1607A.9 Reduction in live loads. Except for roof uniform live loads, all other minimum uniformly distributed live loads, L_o , in Table 1607A.1 are permitted to be reduced in accordance with Section 1607A.9.1 or 1607A.9.2.

1607A.9.1 General. Subject to the limitations of Sections 1607A.9.1.1 through 1607A.9.1.4, members for which a value of $K_{LL}A_T$ is 400 square feet (37.16 m²) or more are permitted to be designed for a reduced live load in accordance with the following equation:

$$L = L_o \left(0.25 + \frac{15}{\sqrt{K_{LL} A_T}} \right)$$

(Equation 16A-24)

For SI:

$$L = L_o \left(0.25 + \frac{457}{\sqrt{K_{LL} A_T}} \right)$$

where:

L = Reduced design live load per square foot (meter) of area supported by the member.

L_o = Unreduced design live load per square foot (meter) of area supported by the member (see Table 1607A.1).

K_{LL} = Live load element factor (see Table 1607A.9.1).

A_T = Tributary area, in square feet (square meters). L shall not be less than $0.50L_o$ for members supporting one floor and L shall not be less than $0.40L_o$ for members supporting two or more floors.

TABLE 1607A.9.1 LIVE LOAD ELEMENT FACTOR, K_{LL}

ELEMENT	K_{LL}
Interior columns	4
Exterior columns without cantilever slabs	4
Edge columns with cantilever slabs	3
Corner columns with cantilever slabs	2
Edge beams without cantilever slabs	2
Interior beams	2
All other members not identified above including:	1

Edge beams with cantilever slabs Cantilever beams Two-way slabs Members without provisions for continuous shear transfer normal to their span	
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1607A.9.1.1 Heavy live loads. Live loads that exceed 100 psf (4.79 kN/m²) shall not be reduced.

Exceptions:

1. The live loads for members supporting two or more floors are permitted to be reduced by a maximum of 20 percent, but the live load shall not be less than *L* as calculated in Section 1607A.9.1.
2. For uses other than storage, where approved, additional live load reductions shall be permitted where shown by the registered design professional that a rational approach has been used and that such reductions are warranted.

1607A.9.1.2 Passenger vehicle garages. The live loads shall not be reduced in passenger vehicle garages except the live loads for members supporting two or more floors are permitted to be reduced by a maximum of 20 percent, but the live load shall not be less than *L* as calculated in Section 1607A.9.1.

1607A.9.1.3 Special occupancies. Live loads of 100 psf (4.79 kN/m²) or less shall not be reduced in public assembly occupancies.

1607A.9.1.4 Special structural elements. Live loads shall not be reduced for one-way slabs except as permitted in Section 1607A.9.1.1. Live loads of 100 psf (4.79 kN/m²) or less shall not be reduced for roof members except as specified in Section 1607A.11.2.

1607A.9.2 Alternate floor live load reduction. As an alternative to Section 1607A.9.1, floor live loads are permitted to be reduced in accordance with the following provisions. Such reductions shall apply to slab systems, beams, girders, columns, piers, walls and foundations.

1. A reduction shall not be permitted in Group A occupancies.
2. A reduction shall not be permitted where the live load exceeds 100 psf (4.79 kN/m²) except that the design live load for members supporting two or more floors is permitted to be reduced by 20 percent.
3. A reduction shall not be permitted in passenger vehicle parking garages except the live loads for members supporting two or more floors are permitted to be reduced by a maximum of 20 percent.
4. For live loads not exceeding 100 psf (4.79 kN/m²), the design live load for any structural member supporting 150 square feet (13.94 m²) or more is permitted to be reduced in accordance with the following equation:

$$R = 0.08 (A - 150)$$

(Equation 16A-25)

For SI: $R = 0.861 (A - 13.94)$

Such reduction shall not exceed the smallest of:

1. 40 percent for horizontal members,
2. 60 percent for vertical members, or
3. *R* as determined by the following equation.

$$R = 23.1 (1 + D/L_o)$$

(Equation 16A-26)

where:

A = Area of floor supported by the member, square feet (m^2).

D = Dead load per square foot (m^2) of area supported.

L_o = Unreduced live load per square foot (m^2) of area supported.

R = Reduction in percent.

1607A.10 Distribution of floor loads. Where uniform floor live loads are involved in the design of structural members arranged so as to create continuity, the minimum applied loads shall be the full dead loads on all spans in combination with the floor live loads on spans selected to produce the greatest effect at each location under consideration. It shall be permitted to reduce floor live loads in accordance with Section 1607A.9.

1607A.11 Roof loads. The structural supports of roofs and marquees shall be designed to resist wind and, where applicable, snow and earthquake loads, in addition to the dead load of construction and the appropriate live loads as prescribed in this section, or as set forth in Table 1607A.1. The live loads acting on a sloping surface shall be assumed to act vertically on the horizontal projection of that surface.

1607A.11.1 Distribution of roof loads. Where uniform roof live loads are reduced to less than 20 psf (0.96 kN/m^2) in accordance with Section 1607A.11.2.1 and are involved in the design of structural members arranged so as to create continuity, the minimum applied loads shall be the full dead loads on all spans in combination with the roof live loads on adjacent spans or on alternate spans, whichever produces the greatest effect. See Section 1607A.11.2 for minimum roof live loads and Section 7.5 of ASCE 7 for partial snow loading.

1607A.11.2 Reduction in roof live loads. The minimum uniformly distributed roof live loads, L_o , in Table 1607A.1 are permitted to be reduced according to the following provisions.

1607A.11.2.1 Flat, pitched and curved roofs. Ordinary flat, pitched and curved roofs are permitted to be designed for a reduced roof live load as specified in the following equation or other controlling combinations of loads in Section 1605A, whichever produces the greater load. In structures where special scaffolding is used as a work surface for workers and materials during maintenance and repair operations, a lower roof load than specified in the following equation shall not be used unless approved by the building official. Greenhouses shall be designed for a minimum roof live load of 12 psf (0.58 kN/m^2).

$$L_r = L_o R_1 R_2$$

(Equation 16A-27)

where: $12 \leq L_r \leq 20$

For SI: $L_r = L_o R_1 R_2$

where: $0.58 \leq L_r \leq 0.96$

L_r = Reduced live load per square foot (m^2) of horizontal projection in pounds per square foot (kN/m^2).

The reduction factors R_1 and R_2 shall be determined as follows:

$$R_1 = 1 \text{ for } A_t \leq 200 \text{ square feet (18.58 } m^2)$$

(Equation 16A-28)

$$R_1 = 1.2 - 0.001A_t \text{ for } 200 \text{ square feet} < A_t < 600 \text{ square feet}$$

(Equation 16A-29)

For SI: $1.2 - 0.011A_t$, for 18.58 square meters $< A_t < 55.74$ square meters

$$R_1 = 0.6 \text{ for } A_t > 600 \text{ square feet (55.74 } m^2)$$

(Equation 16A-30)

where:

A_t = Tributary area (span length multiplied by effective width) in square feet (m^2) supported by any structural member, and

$R_2 = 1$ for $F \leq 4$
(Equation 16A-31)

$R_2 = 1.2 - 0.05 F$ for $4 < F < 12$
(Equation 16A-32)

$R_2 = 0.6$ for $F > 12$
(Equation 16A-33)

where:

F = For a sloped roof, the number of inches of rise per foot (for SI: $F = 0.12 \times$ slope, with slope expressed as a percentage), or for an arch or dome, the rise-to-span ratio multiplied by 32.

1607A.11.2.2 Special-purpose roofs. Roofs used for promenade purposes, roof gardens, assembly purposes or other special purposes shall be designed for a minimum live load as required in Table 1607A.1. Such roof live loads are permitted to be reduced in accordance with 1607A.9. *(Relocated from 1607A.4.4, 2001 CBC) Uncovered open-frame roof structures shall be designed for a vertical live load of not less than 10 pounds per square foot (0.48 kN/m²) of the total area encompassed by the framework.*

1607A.11.2.3 Landscaped roofs. Where roofs are to be landscaped, the uniform design live load in the landscaped area shall be 20 psf (0.958 kN/m²). The weight of the landscaping materials shall be considered as dead load and shall be computed on the basis of saturation of the soil.

1607A.11.2.4 Awnings and canopies. Awnings and canopies shall be designed for uniform live loads as required in Table 1607A.1 as well as for snow loads and wind loads as specified in Sections 1608A and 1609A.

1607A.12 Crane loads. The crane live load shall be the rated capacity of the crane. Design loads for the runway beams, including connections and support brackets, of moving bridge cranes and monorail cranes shall include the maximum wheel loads of the crane and the vertical impact, lateral and longitudinal forces induced by the moving crane.

1607A.12.1 Maximum wheel load. The maximum wheel loads shall be the wheel loads produced by the weight of the bridge, as applicable, plus the sum of the rated capacity and the weight of the trolley with the trolley positioned on its runway at the location where the resulting load effect is maximum.

1607A.12.2 Vertical impact force. The maximum wheel loads of the crane shall be increased by the percentages shown below to determine the induced vertical impact or vibration force:

Monorail cranes (powered)	25 percent
Cab-operated or remotely operated bridge cranes (powered)	25 percent
Pendant-operated bridge cranes (powered)	10 percent
Bridge cranes or monorail cranes with hand-gear bridge, trolley and hoist.....	0 percent

1607A.12.3 Lateral force. The lateral force on crane runway beams with electrically powered trolleys shall be calculated as 20 percent of the sum of the rated capacity of the crane and the weight of the hoist and trolley. The lateral force shall be assumed to act horizontally at the traction surface of a runway beam, in either direction perpendicular to the beam, and shall be distributed according to the lateral stiffness of the runway beam and supporting structure.

1607A.12.4 Longitudinal force. The longitudinal force on crane runway beams, except for bridge cranes with hand-gear bridges, shall be calculated as 10 percent of the maximum wheel loads of the crane. The longitudinal force shall be assumed to act horizontally at the traction surface of a runway beam, in either direction parallel to the beam.

1607A.13 Interior walls and partitions. Interior walls and partitions that exceed 6 feet (1829 mm) in height, including their finish materials, shall have adequate strength to resist the loads to which they are subjected but not less than a horizontal load of 5 psf (0.240 kN/m²). *(Relocated from 1611A.5, 2001 CBC) The 5 psf (0.24 kN/m²) load need not be applied simultaneously with wind or seismic loads. The deflection of such walls under a load of 5 psf (0.24 kN/m²) shall not exceed 1 / 240 of the span for walls with brittle finishes and 1 / 120 of the span for walls with flexible finishes.*

Exception: Fabric partitions complying with Section 1607A.13.1 shall not be required to resist the minimum horizontal load of 5 psf (0.24 kN/m²).

1607A.13.1 Fabric partitions. Fabric partitions that exceed 6 feet (1829 mm) in height, including their finish materials, shall have adequate strength to resist the following load conditions:

1. A horizontal distributed load of 5 psf (0.24 kN/m²) applied to the partition framing. The total area used to determine the distributed load shall be the area of the fabric face between the framing members to which the fabric is attached. The total distributed load shall be uniformly applied to such framing members in proportion to the length of each member.
2. A concentrated load of 40 pounds (0.176 kN) applied to an 8-inch diameter (203 mm) area [50.3 square inches (32 452 mm²)] of the fabric face at a height of 54 inches (1372 mm) above the floor.

SECTION 1608A SNOW LOADS

1608A.1 General. Design snow loads shall be determined in accordance with Chapter 7 of ASCE 7, but the design roof load shall not be less than that determined by Section 1607A.

1608A.2 Ground snow loads. The ground snow loads to be used in determining the design snow loads for roofs shall be determined in accordance with ASCE 7 or Figure 1608A.2 for the contiguous United States and Table 1608.2 for Alaska. Site-specific case studies shall be made in areas designated "CS" in Figure 1608A.2. Ground snow loads for sites at elevations above the limits indicated in Figure 1608A.2 and for all sites within the CS areas shall be approved. Ground snow load determination for such sites shall be based on an extreme value statistical analysis of data available in the vicinity of the site using a value with a 2-percent annual probability of being exceeded (50-year mean recurrence interval). Snow loads are zero for Hawaii, except in mountainous regions as approved by the building official.

1608A.3 [For DSA-SS] Determination of snow loads. *The ground snow load or the design snow load for roofs shall conform with the adopted ordinance of the city, county, or city and county in which the project site is located, and shall be approved by DSA.*

TABLE 1608.2 – GROUND SNOW LOADS, pg. , FOR ALASKAN LOCATIONS

LOCATION	POUNDS PER SQUARE FOOT	LOCATION	POUNDS PER SQUARE FOOT	LOCATION	POUNDS PER SQUARE FOOT
Adak	30	Galena	60	Petersburg	150
Anchorage	50	Gulkana	70	St. Paul Islands	40
Angeon	70	Homer	40	Seward	50
Barrow	25	Juneau	60	Shemya	25
Barter Island	35	Kenai	70	Sitka	50
Bethel	40	Kodiak	30	Talkeetna	120

Big Delta	50	Kotzebue	60	Unalakleet	50
Cold Bay	25	McGrath	70	Valdez	160
Cordova	100	Nenana	80	Whittier	300
Fairbanks	60	Nome	70	Wrangell	60
Fort Yukon	60	Palmer	50	Yakutat	150

For SI: 1 pound per square foot = 0.0479 kN/m².

(Figure 1608.A.2 is identical to 2006 IBC Figure 1608.2)

FIGURE 1608.A.2 - GROUND SNOW LOADS, p_g , FOR THE UNITED STATES (psf)

(Figure 1608.A.2 is identical to 2006 IBC Figure 1608.2)

FIGURE 1608.A.2 - continued - GROUND SNOW LOADS, p_g , FOR THE UNITED STATES (psf)

SECTION 1609.A WIND LOADS

(Figure 1609A BASIC WIND SPEED (3-SECOND GUST) is identical to 2006 IBC Figure 1609 BASIC WIND SPEED (3-SECOND GUST))

FIGURE 1609.A - BASIC WIND SPEED (3-SECOND GUST)

(Figure 1609A — continued - BASIC WIND SPEED (3-SECOND GUST) is identical to 2006 IBC Figure 1609—continued BASIC WIND SPEED (3-SECOND GUST))

FIGURE 1609.A — continued - BASIC WIND SPEED (3-SECOND GUST)

(Figure 1609A -continued BASIC WIND SPEED (3-SECOND GUST) WESTERN GULF OF MEXICO HURRICANE COASTLINE is identical to 2006 IBC Figure 1609 -continued BASIC WIND SPEED (3-SECOND GUST) WESTERN GULF OF MEXICO HURRICANE COASTLINE)

FIGURE 1609A- continued - BASIC WIND SPEED (3-SECOND GUST) WESTERN GULF OF MEXICO HURRICANE COASTLINE

(Figure 1609A -continued BASIC WIND SPEED (3-SECOND GUST) EASTERN GULF OF MEXICO AND SOUTHEASTERN U.S. HURRICANE COASTLINE is identical to 2006 IBC Figure 1609 -continued BASIC WIND SPEED (3-SECOND GUST) EASTERN GULF OF MEXICO AND SOUTHEASTERN U.S. HURRICANE COASTLINE)

FIGURE 1609A- continued - BASIC WIND SPEED (3-SECOND GUST) EASTERN GULF OF MEXICO AND SOUTHEASTERN U.S. HURRICANE COASTLINE

(Figure 1609A -continued BASIC WIND SPEED (3-SECOND GUST) MID AND NORTHERN ATLANTIC HURRICANE COASTLINE is identical to 2006 IBC Figure 1609 -continued BASIC WIND SPEED (3-SECOND GUST) MID AND NORTHERN ATLANTIC HURRICANE COASTLINE)

FIGURE 1609A – continued - BASIC WIND SPEED (3-SECOND GUST) MID AND NORTHERN ATLANTIC HURRICANE COASTLINE

1609A.1 Applications. Buildings, structures and parts thereof shall be designed to withstand the minimum wind loads prescribed herein. Decreases in wind loads shall not be made for the effect of shielding by other structures.

1609A.1.1 Determination of wind loads. Wind loads on every building or structure shall be determined in accordance with Chapter 6 of ASCE 7. The type of opening protection required, the basic wind speed and the exposure category for a site is permitted to be determined in accordance with Section 1609A or ASCE 7. Wind shall be assumed to come from any horizontal direction and wind pressures shall be assumed to act normal to the surface considered.

Exceptions:

1. Subject to the limitations of Section 1609A.1.1.1, the provisions of SBCCI SSTD 10 shall be permitted for applicable Group R-2 and R-3 buildings.
2. Subject to the limitations of Section 1609A.1.1.1, residential structures using the provisions of the AF&PA WFCM.
3. Designs using NAAMM FP 1001.
4. Designs using TIA/EIA-222 for antenna-supporting structures and antennas.

1609A.1.1.1 Applicability. The provisions of SSTD 10 are applicable only to buildings located within Exposure B or C as defined in Section 1609A.4. The provisions of SBCCI SSTD 10 and the AF&PA WFCM shall not apply to buildings sited on the upper half of an isolated hill, ridge or escarpment meeting the following conditions:

1. The hill, ridge or escarpment is 60 feet (18 288 mm) or higher if located in Exposure B or 30 feet (9144 mm) or higher if located in Exposure C;
2. The maximum average slope of the hill exceeds 10 percent; and

- The hill, ridge or escarpment is unobstructed upwind by other such topographic features for a distance from the high point of 50 times the height of the hill or 1 mile (1.61 km), whichever is greater.

1609A.1.1.2 [For DSA – SS] Special Wind Regions. The basic wind speed for projects located in special wind regions as defined in Figure 1609A shall conform with the adopted ordinance of the city, county, or city and county in which the project site is located, and shall be approved by DSA.

1609A.1.2 Protection of openings. In wind-borne debris regions, glazing in buildings shall be impact-resistant or protected with an impact-resistant covering meeting the requirements of an approved impact-resisting standard or ASTM E 1996 and ASTM E 1886 referenced therein as follows:

- Glazed openings located within 30 feet (9144 mm) of grade shall meet the requirements of the Large Missile Test of ASTM E 1996.
- Glazed openings located more than 30 feet (9144 mm) above grade shall meet the provisions of the Small Missile Test of ASTM E 1996.

Exceptions:

- Wood structural panels with a minimum thickness of $\frac{7}{16}$ inch (11.1 mm) and maximum panel span of 8 feet (2438 mm) shall be permitted for opening protection in one- and two-story buildings. Panels shall be pre-cut so that they shall be attached to the framing surrounding the opening containing the product with the glazed opening. Panels shall be secured with the attachment hardware provided. Attachments shall be designed to resist the components and cladding loads determined in accordance with the provisions of ASCE 7. Attachment in accordance with Table 1609A.1.2 is permitted for buildings with a mean roof height of 33 feet (10 058 mm) or less where wind speeds do not exceed 130 mph (57.2 m/s).
- Glazing in Occupancy Category I buildings as defined in Section 1604A.5, including greenhouses that are occupied for growing plants on a production or research basis, without public access shall be permitted to be unprotected.
- Glazing in Occupancy Category II, III or IV buildings located over 60 feet (18 288 mm) above the ground and over 30 feet (9144 mm) above aggregate surface roofs located within 1,500 feet (458 m) of the building shall be permitted to be unprotected.

TABLE 1609A.1.2 - WIND-BORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANELS^{a,b,c,d}

FASTENER TYPE	FASTENER SPACING (inches)		
	Panel Span ≤ 4 feet	4 feet < Panel Span ≤ 6 feet	6 feet < Panel Span ≤ 8 feet
No. 6 screws	16	12	9
No. 8 screws	16	16	12

For SI: 1 inch = 25.4 mm,
 1 foot = 304.8 mm,
 1 pound = 4.4 N,
 1 mile per hour = 0.44 m/s.

- This table is based on a maximum wind speed (3-second gust) of 130 mph and mean roof height of 33 feet or less.
- Fasteners shall be installed at opposing ends of the wood structural panel. Fasteners shall be located a minimum of 1 inch from the edge of the panel.
- Fasteners shall be long enough to penetrate through the exterior wall covering a minimum of 1.75 inches into wood wall framing; a minimum of 1.25 inches into concrete block or concrete; or into steel framing by at least three threads. Fasteners shall be located a minimum of 2.5 inches from the edge of concrete block or concrete.
- Where screws are attached to masonry or masonry/stucco, they shall be attached utilizing vibration-resistant anchors having a

minimum withdrawal capacity of 490 pounds.

1609A.1.2.1 Louvers. Louvers protecting intake and exhaust ventilation ducts not assumed to be open that are located within 30 feet (9144 mm) of grade shall meet requirements of an approved impact-resisting standard or the Large Missile Test of ASTM E 1996.

1609A.1.3 (Relocated from 1620A, 2001 CBC) Story Drift for Wind Loads. *The calculated story drift due to wind pressures shall not exceed 0.005 times the story height for buildings less than 65 feet (19,812 mm) in height or 0.004 times the story height for buildings 65 feet (19,812 mm) or greater in height.*

1609A.2 Definitions. The following words and terms shall, for the purposes of Section 1609A, have the meanings shown herein.

HURRICANE-PRONE REGIONS. Areas vulnerable to hurricanes defined as:

1. The U. S. Atlantic Ocean and Gulf of Mexico coasts where the basic wind speed is greater than 90 mph (40 m/s) and
2. Hawaii, Puerto Rico, Guam, Virgin Islands and American Samoa.

WIND-BORNE DEBRIS REGION. Portions of hurricane-prone regions that are within 1 mile (1.61 km) of the coastal mean high water line where the basic wind speed is 110 mph (48 m/s) or greater; or portions of hurricane-prone regions where the basic wind speed is 120 mph (53 m/s) or greater; or Hawaii.

1609A.3 Basic wind speed. The basic wind speed, in mph, for the determination of the wind loads shall be determined by Figure 1609A. Basic wind speed for the special wind regions indicated, near mountainous terrain and near gorges shall be in accordance with local jurisdiction requirements. Basic wind speeds determined by the local jurisdiction shall be in accordance with Section 6.5.4 of ASCE 7.

In nonhurricane-prone regions, when the basic wind speed is estimated from regional climatic data, the basic wind speed shall be not less than the wind speed associated with an annual probability of 0.02 (50-year mean recurrence interval), and the estimate shall be adjusted for equivalence to a 3-second gust wind speed at 33 feet (10 m) above ground in Exposure Category C. The data analysis shall be performed in accordance with Section 6.5.4.2 of ASCE 7.

1609A.3.1 Wind speed conversion. When required, the 3-second gust basic wind speeds of Figure 1609A shall be converted to fastest-mile wind speeds, V_{fm} , using Table 1609A.3.1 or Equation 16A-34.

$$V_{fm} = \frac{(V_{3s} - 10.5)}{1.05}$$

(Equation 16A-34)

where:

V_{3s} = 3-second gust basic wind speed from Figure 1609A.

TABLE 1609A.3.1 - EQUIVALENT BASIC WIND SPEEDS^{a,b,c}

V_{3s}	85	90	100	105	110	120	125	130	140	145	150	160	170
V_{fm}	71	76	85	90	95	104	109	114	123	128	133	142	152

For SI: 1 mile per hour = 0.44 m/s.

- a. Linear interpolation is permitted.
- b. V_{3S} is the 3-second gust wind speed (mph).
- c. V_{fm} is the fastest mile wind speed (mph).

1609A.4 Exposure category. For each wind direction considered, an exposure category that adequately reflects the characteristics of ground surface irregularities shall be determined for the site at which the building or structure is to be constructed. Account shall be taken of variations in ground surface roughness that arise from natural topography and vegetation as well as from constructed features.

Exception: *(Relocated from 1619A, CBC 2001) The wind design shall comply with Exposure C requirements unless the architect or structural engineer in general responsible charge can justify to the enforcement agency that the building site and surrounding terrain conform to the criteria for Exposure B. Minimum data to establish the exposure category shall be a topographic map (e.g., United States Geological Survey quadrangle maps) and aerial photographs except Exception: that for Exposure B sites located within urban areas, a vicinity map of sufficient size and scale to verify compliance may be provided.*

1609A.4.1 Wind directions and sectors. For each selected wind direction at which the wind loads are to be evaluated, the exposure of the building or structure shall be determined for the two upwind sectors extending 45 degrees (0.79 rad) either side of the selected wind direction. The exposures in these two sectors shall be determined in accordance with Sections 1609A.4.2 and 1609A.4.3 and the exposure resulting in the highest wind loads shall be used to represent winds from that direction.

1609A.4.2 Surface roughness categories. A ground surface roughness within each 45-degree (0.79 rad) sector shall be determined for a distance upwind of the site as defined in Section 1609A.4.3 from the categories defined below, for the purpose of assigning an exposure category as defined in Section 1609A.4.3.

Surface Roughness B. Urban and suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

Surface Roughness C. Open terrain with scattered obstructions having heights generally less than 30 feet (9144 mm). This category includes flat open country, grasslands, and all water surfaces in hurricane-prone regions.

Surface Roughness D. Flat, unobstructed areas and water surfaces outside hurricane-prone regions. This category includes smooth mud flats, salt flats and unbroken ice.

1609A.4.3 Exposure categories. An exposure category shall be determined in accordance with the following:

Exposure B. Exposure B shall apply where the ground surface roughness condition, as defined by Surface Roughness B, prevails in the upwind direction for a distance of at least 2,600 feet (792 m) or 20 times the height of the building, whichever is greater.

Exception: For buildings whose mean roof height is less than or equal to 30 feet (9144 mm), the upwind distance is permitted to be reduced to 1,500 feet (457 m).

Exposure C. Exposure C shall apply for all cases where Exposures B or D do not apply.

Exposure D. Exposure D shall apply where the ground surface roughness, as defined by Surface Roughness D, prevails in the upwind direction for a distance of at least 5,000 feet (1524 m) or 20 times the height of the

building, whichever is greater. Exposure D shall extend inland from the shoreline for a distance of 600 feet (183 m) or 20 times the height of the building, whichever is greater.

1609A.5 Roof systems.

1609A.5.1 Roof deck. The roof deck shall be designed to withstand the wind pressures determined in accordance with ASCE 7.

1609A.5.2 Roof coverings. Roof coverings shall comply with Section 1609A.5.1.

Exception: Rigid tile roof coverings that are air permeable and installed over a roof deck complying with Section 1609A.5.1 are permitted to be designed in accordance with Section 1609A.5.3.

Asphalt shingles installed over a roof deck complying with Section 1609A.5.1 shall be tested to determine the resistance of the sealant to uplift forces using ASTM D 6381.

Asphalt shingles installed over a roof deck complying with Section 1609A.5.1 are permitted to be designed using UL 2390 to determine appropriate uplift and force coefficients applied to the shingle.

1609A.5.3 Rigid tile. Wind loads on rigid tile roof coverings shall be determined in accordance with the following equation:

$$M_a = q_h C_L b L L_a [1.0 - GC_p]$$

(Equation 16A-35)

$$\text{For SI: } M_a = \frac{q_h C_L b L L_a [1.0 - GC_p]}{1,000}$$

where:

b = Exposed width, feet (mm) of the roof tile.

C_L = Lift coefficient. The lift coefficient for concrete and clay tile shall be 0.2 or shall be determined by test in accordance with Section 1715A.2.

GC_p = Roof pressure coefficient for each applicable roof zone determined from Chapter 6 of ASCE 7. Roof coefficients shall not be adjusted for internal pressure.

L = Length, feet (mm) of the roof tile.

L_a = Moment arm, feet (mm) from the axis of rotation to the point of uplift on the roof tile. The point of uplift shall be taken at $0.76L$ from the head of the tile and the middle of the exposed width. For roof tiles with nails or screws (with or without a tail clip), the axis of rotation shall be taken as the head of the tile for direct deck application or as the top edge of the batten for battened applications. For roof tiles fastened only by a nail or screw along the side of the tile, the axis of rotation shall be determined by testing. For roof tiles installed with battens and fastened only by a clip near the tail of the tile, the moment arm shall be determined about the top edge of the batten with consideration given for the point of rotation of the tiles based on straight bond or broken bond and the tile profile.

M_a = Aerodynamic uplift moment, feet-pounds (N-mm) acting to raise the tail of the tile.

q_h = Wind velocity pressure, psf (kN/m²) determined from Section 6.5.10 of ASCE 7.

Concrete and clay roof tiles complying with the following limitations shall be designed to withstand the aerodynamic uplift moment as determined by this section.

1. The roof tiles shall be either loose laid on battens, mechanically fastened, mortar set or adhesive set.
2. The roof tiles shall be installed on solid sheathing which has been designed as components and cladding.
3. An underlayment shall be installed in accordance with Chapter 15.
4. The tile shall be single lapped interlocking with a minimum head lap of not less than 2 inches (51 mm).
5. The length of the tile shall be between 1.0 and 1.75 feet (305 mm and 533 mm).
6. The exposed width of the tile shall be between 0.67 and 1.25 feet (204 mm and 381 mm).
7. The maximum thickness of the tail of the tile shall not exceed 1.3 inches (33 mm).
8. Roof tiles using mortar set or adhesive set systems shall have at least two-thirds of the tile's area free of mortar or adhesive contact.

SECTION 1610A - SOIL LATERAL LOAD

1610A.1 General. Basement, foundation and retaining walls shall be designed to resist lateral soil loads. Soil loads specified in Table 1610A.1 shall be used as the minimum design lateral soil loads unless specified otherwise in a soil investigation report approved by the building official. Basement walls and other walls in which horizontal movement is restricted at the top shall be designed for at-rest pressure. Retaining walls free to move and rotate at the top are permitted to be designed for active pressure. Design lateral pressure from surcharge loads shall be added to the lateral earth pressure load. Design lateral pressure shall be increased if soils with expansion potential are present at the site.

Exception: Basement walls extending not more than 8 feet (2438 mm) below grade and supporting flexible floor systems shall be permitted to be designed for active pressure.

TABLE 1610A.1 - SOIL LATERAL LOAD

DESCRIPTION OF BACKFILL MATERIAL ^c	UNIFIED SOIL CLASSIFICATION	DESIGN LATERAL SOIL LOAD ^a (pound per square foot per foot of depth)	
		Active pressure	At-rest pressure
Well-graded, clean gravels; gravel-sand mixes	GW	30	60
Poorly graded clean gravels; gravel-sand mixes	GP	30	60
Silty gravels, poorly graded gravel-sand mixes	GM	40	60
Clayey gravels, poorly graded gravel-and-clay mixes	GC	45	60
Well-graded, clean sands; gravelly sand mixes	SW	30	60
Poorly graded clean sands; sand-gravel mixes	SP	30	60
Silty sands, poorly graded sand-silt mixes	SM	45	60
Sand-silt clay mix with plastic fines	SM-SC	45	100
Clayey sands, poorly graded sand-clay mixes	SC	60	100
Inorganic silts and clayey silts	ML	45	100
Mixture of inorganic silt and clay	ML-CL	60	100
Inorganic clays of low to medium plasticity	CL	60	100
Organic silts and silt clays, low plasticity	OL	Note b	Note b
Inorganic clayey silts, elastic silts	MH	Note b	Note b
Inorganic clays of high plasticity	CH	Note b	Note b
Organic clays and silty clays	OH	Note b	Note b

For SI: 1 pound per square foot per foot of depth = 0.157 kPa/m, 1 foot = 304.8 mm.

- a. Design lateral soil loads are given for moist conditions for the specified soils at their optimum densities. Actual field conditions shall govern. Submerged or saturated soil pressures shall include the weight of the buoyant soil plus the hydrostatic loads.
- b. Unsuitable as backfill material.
- c. The definition and classification of soil materials shall be in accordance with ASTM D 2487.

SECTION 1611A - RAIN LOADS

1611A.1 Design rain loads. Each portion of a roof shall be designed to sustain the load of rainwater that will accumulate on it if the primary drainage system for that portion is blocked plus the uniform load caused by water that rises above the inlet of the secondary drainage system at its design flow.

$$R = 5.2 (d_s + d_h)$$

(Equation 16A-36)

For SI: $R = 0.0098 (d_s + d_h)$

where:

d_h = Additional depth of water on the undeflected roof above the inlet of secondary drainage system at its design flow (i.e., the hydraulic head), in inches (mm).

d_s = Depth of water on the undeflected roof up to the inlet of secondary drainage system when the primary drainage system is blocked (i.e., the static head), in inches (mm).

R = Rain load on the undeflected roof, in psf (kN/m²). When the phrase "undeflected roof" is used, deflections from loads (including dead loads) shall not be considered when determining the amount of rain on the roof.

1611A.2 Ponding instability. For roofs with a slope less than $\frac{1}{4}$ inch per foot [1.19 degrees (0.0208 rad)], the design calculations shall include verification of adequate stiffness to preclude progressive deflection in accordance with Section 8.4 of ASCE 7.

1611A.3 Controlled drainage. Roofs equipped with hardware to control the rate of drainage shall be equipped with a secondary drainage system at a higher elevation that limits accumulation of water on the roof above that elevation. Such roofs shall be designed to sustain the load of rainwater that will accumulate on them to the elevation of the secondary drainage system plus the uniform load caused by water that rises above the inlet of the secondary drainage system at its design flow determined from Section 1611A.1. Such roofs shall also be checked for ponding instability in accordance with Section 1611A.2.

SECTION 1612A - FLOOD LOADS

1612A.1 General. Within flood hazard areas as established in Section 1612A.3, all new construction of buildings, structures and portions of buildings and structures, including substantial improvement and restoration of substantial damage to buildings and structures, shall be designed and constructed to resist the effects of flood hazards and flood loads. For buildings that are located in more than one flood hazard area, the provisions associated with the most restrictive flood hazard area shall apply.

1612A.2 Definitions. The following words and terms shall, for the purposes of this section, have the meanings shown herein.

BASE FLOOD. The flood having a 1-percent chance of being equaled or exceeded in any given year.

BASE FLOOD ELEVATION. The elevation of the base flood, including wave height, relative to the National Geodetic Vertical Datum (NGVD), North American Vertical Datum (NAVD) or other datum specified on the Flood Insurance Rate Map (FIRM).

BASEMENT. The portion of a building having its floor subgrade (below ground level) on all sides.

DESIGN FLOOD. The flood associated with the greater of the following two areas:

1. Area with a flood plain subject to a 1-percent or greater chance of flooding in any year; or
2. Area designated as a flood hazard area on a community's flood hazard map, or otherwise legally designated.

DESIGN FLOOD ELEVATION. The elevation of the "design flood," including wave height, relative to the datum specified on the community's legally designated flood hazard map. In areas designated as Zone AO, the design flood elevation shall be the elevation of the highest existing grade of the building's perimeter plus the depth number (in feet) specified on the flood hazard map. In areas designated as Zone AO where a depth number is not specified on the map, the depth number shall be taken as being equal to 2 feet (610 mm).

DRY FLOODPROOFING. A combination of design modifications that results in a building or structure, including the attendant utility and sanitary facilities, being water tight with walls substantially impermeable to the passage of water and with structural components having the capacity to resist loads as identified in ASCE 7.

EXISTING CONSTRUCTION. Any buildings and structures for which the "start of construction" commenced before the effective date of the community's first flood plain management code, ordinance or standard. "Existing construction" is also referred to as "existing structures."

EXISTING STRUCTURE. See "Existing construction."

FLOOD or FLOODING. A general and temporary condition of partial or complete inundation of normally dry land from:

1. The overflow of inland or tidal waters.
2. The unusual and rapid accumulation or runoff of surface waters from any source.

FLOOD DAMAGE-RESISTANT MATERIALS. Any construction material capable of withstanding direct and prolonged contact with floodwaters without sustaining any damage that requires more than cosmetic repair.

FLOOD HAZARD AREA. The greater of the following two areas:

1. The area within a flood plain subject to a 1-percent or greater chance of flooding in any year.
2. The area designated as a flood hazard area on a community's flood hazard map, or otherwise legally designated.

FLOOD HAZARD AREA SUBJECT TO HIGH VELOCITY WAVE ACTION. Area within the flood hazard area that is subject to high velocity wave action, and shown on a Flood Insurance Rate Map (FIRM) or other flood hazard map as Zone V, VO, VE or V1-30.

FLOOD INSURANCE RATE MAP (FIRM). An official map of a community on which the Federal Emergency Management Agency (FEMA) has delineated both the special flood hazard areas and the risk premium zones applicable to the community.

FLOOD INSURANCE STUDY. The official report provided by the Federal Emergency Management Agency containing the Flood Insurance Rate Map (FIRM), the Flood Boundary and Floodway Map (FBFM), the water surface elevation of the base flood and supporting technical data.

FLOODWAY. The channel of the river, creek or other watercourse and the adjacent land areas that must be reserved in order to discharge the base flood without cumulatively increasing the water surface elevation more than a designated height.

LOWEST FLOOR. The floor of the lowest enclosed area, including basement, but excluding any unfinished or flood-resistant enclosure, usable solely for vehicle parking, building access or limited storage provided that such enclosure is not built so as to render the structure in violation of this section.

SPECIAL FLOOD HAZARD AREA. The land area subject to flood hazards and shown on a Flood Insurance Rate

Map or other flood hazard map as Zone A, AE, A1-30, A99, AR, AO, AH, V, VO, VE or V1-30.

START OF CONSTRUCTION. The date of permit issuance for new construction and substantial improvements to existing structures, provided the actual start of construction, repair, reconstruction, rehabilitation, addition, placement or other improvement is within 180 days after the date of issuance. The actual start of construction means the first placement of permanent construction of a building (including a manufactured home) on a site, such as the pouring of a slab or footings, installation of pilings or construction of columns.

Permanent construction does not include land preparation (such as clearing, excavation, grading or filling), the installation of streets or walkways, excavation for a basement, footings, piers or foundations, the erection of temporary forms or the installation of accessory buildings such as garages or sheds not occupied as dwelling units or not part of the main building. For a substantial improvement, the actual "start of construction" means the first alteration of any wall, ceiling, floor or other structural part of a building, whether or not that alteration affects the external dimensions of the building.

SUBSTANTIAL DAMAGE. Damage of any origin sustained by a structure whereby the cost of restoring the structure to its before-damaged condition would equal or exceed 50 percent of the market value of the structure before the damage occurred.

SUBSTANTIAL IMPROVEMENT. Any repair, reconstruction, rehabilitation, addition or improvement of a building or structure, the cost of which equals or exceeds 50 percent of the market value of the structure before the improvement or repair is started. If the structure has sustained substantial damage, any repairs are considered substantial improvement regardless of the actual repair work performed. The term does not, however, include either:

1. Any project for improvement of a building required to correct existing health, sanitary or safety code violations identified by the building official and that are the minimum necessary to assure safe living conditions.
2. Any alteration of a historic structure provided that the alteration will not preclude the structure's continued designation as a historic structure.

1612A.3 Establishment of flood hazard areas. To establish flood hazard areas, the governing body shall adopt a flood hazard map and supporting data. The flood hazard map shall include, at a minimum, areas of special flood hazard as identified by the Federal Emergency Management Agency in an engineering report entitled "The Flood Insurance Study for [INSERT NAME OF JURISDICTION], dated [INSERT DATE OF ISSUANCE] Agency's Flood Insurance Study (FIS) adopted by the local authority having jurisdiction where the project is located." as amended or revised with the accompanying Flood Insurance Rate Map (FIRM) and Flood Boundary and Floodway Map (FBFM) and related supporting data along with any revisions thereto. The adopted flood hazard map and supporting data are hereby adopted by reference and declared to be part of this section.

1612A.4 Design and construction. The design and construction of buildings and structures located in flood hazard areas, including flood hazard areas subject to high velocity wave action, shall be in accordance with ASCE 24.

1612A.5 Flood hazard documentation. The following documentation shall be prepared and sealed by a registered design professional and submitted to the building official:

1. For construction in flood hazard areas not subject to high-velocity wave action:
 - 1.1. The elevation of the lowest floor, including the basement, as required by the lowest floor elevation inspection in Section 109.3.3, Appendix Chapter 1.
 - 1.2. For fully enclosed areas below the design flood elevation where provisions to allow for the automatic entry and exit of floodwaters do not meet the minimum requirements in Section 2.6.2.1 of ASCE 24, construction documents shall include a statement that the design will provide for equalization of hydrostatic flood forces in accordance with Section 2.6.2.2 of ASCE 24.
 - 1.3. For dry floodproofed nonresidential buildings, construction documents shall include a statement that the dry floodproofing is designed in accordance with ASCE 24.
2. For construction in flood hazard areas subject to high-velocity wave action:
 - 2.1. The elevation of the bottom of the lowest horizontal structural member as required by the lowest floor elevation inspection in Section 109.3.3, Appendix Chapter 1.

- 2.2. Construction documents shall include a statement that the building is designed in accordance with ASCE 24, including that the pile or column foundation and building or structure to be attached thereto is designed to be anchored to resist flotation, collapse and lateral movement due to the effects of wind and flood loads acting simultaneously on all building components, and other load requirements of Chapter 16A.
- 2.3. For breakaway walls designed to resist a nominal load of less than 10 psf (0.48 kN/m²) or more than 20 psf (0.96 kN/m²), construction documents shall include a statement that the breakaway wall is designed in accordance with ASCE 24.

SECTION 1613A - EARTHQUAKE LOADS

1613A.1 Scope. Every structure, and portion thereof, including nonstructural components that are permanently attached to structures and their supports and attachments, shall be designed and constructed to resist the effects of earthquake motions in accordance with ASCE 7 *with all the modifications incorporated herein*, excluding Chapter 14 and Appendix 11A. The seismic design category for a structure ~~is permitted to shall~~ be determined in accordance with Section 1613A ~~or ASCE 7~~.

Exceptions:

1. ~~Not permitted by OSHPD and DSA-SS. Detached one- and two-family dwellings, assigned to Seismic Design Category A, B or C, or located where the mapped short period spectral response acceleration, SS, is less than 0.4 g.~~
2. ~~Not permitted by OSHPD and DSA-SS. The seismic force resisting system of wood-frame buildings that conform to the provisions of Section 2308 are not required to be analyzed as specified in this section.~~
3. ~~Not permitted by OSHPD and DSA-SS. Agricultural storage structures intended only for incidental human occupancy.~~
4. Structures that require special consideration of their response characteristics and environment that are not addressed by this code or ASCE 7 and for which other regulations provide seismic criteria, such as vehicular bridges, electrical transmission towers, hydraulic structures, buried utility lines and their appurtenances and nuclear reactors.

1613A.1.1 (Relocated from 1626A.4, 2001 CBC) Configuration. *When the design of a structure, due to the unusual configuration of the structure or parts of the structure, does not provide at least the same safety against earthquake damage as provided by the applicable portions of this section, when applied in the design of a similar structure of customary configuration, framing and assembly of materials, the enforcement agency shall withhold its approval.*

1613A.2 Definitions. The following words and terms shall, for the purposes of this section, have the meanings shown herein. *Definition provided in Section 3402A.1 and ASCE 7 Section 11.2 shall apply when appropriate in addition to terms defined in this section.*

(Relocated from 1641A, 2001 CBC) ACTIVE EARTHQUAKE FAULT. *A fault that has exhibited surface displacement within Holocene time (about 11,000 years) as determined by ~~California Division of Mines & Geology~~ California Geological Survey (CGS) under the Alquist-Priolo Earthquake Fault Zoning Act or other authoritative source, Federal, State or Local Governmental Agency.*

(Relocated from 1627A, 2001 CBC) BASE. *The level at which the horizontal seismic ground motions are considered to be imparted to the structure or the level at which the structure as a dynamic vibrator is supported. This level does not necessarily coincide with the ground level.*

DESIGN EARTHQUAKE GROUND MOTION. The earthquake ground motion that buildings and structures are specifically proportioned to resist in Section 1613A.

(Relocated from 1641A, 2001 CBC) DISTANCE FROM AN ACTIVE EARTHQUAKE FAULT. *Distance measured from the nearest point of the building to the closest edge of an Alquist-Priolo Earthquake fault zone for an active fault, if such a map exists, or to the closest mapped splay of the fault.*

*(Relocated from 1627A, 2001 CBC) **HOSPITAL BUILDINGS.** Hospital buildings and all other medical facilities as defined in Section 1250, Health and Safety Code.*

*(Relocated from 1627A, 2001 CBC) **IRREGULAR STRUCTURE.** A structure designed as having one or more plan or vertical irregularities per ASCE 7 Section 12.3.*

MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION. The most severe earthquake effects considered by this code.

MECHANICAL SYSTEMS. For the purposes of determining seismic loads in ASCE 7, mechanical systems shall include plumbing systems as specified therein.

ORTHOGONAL. To be in two horizontal directions, at 90 degrees (1.57 rad) to each other.

SEISMIC DESIGN CATEGORY. A classification assigned to a structure based on its occupancy category and the severity of the design earthquake ground motion at the site.

SEISMIC-FORCE-RESISTING SYSTEM. That part of the structural system that has been considered in the design to provide the required resistance to the prescribed seismic forces.

SITE CLASS. A classification assigned to a site based on the types of soils present and their engineering properties as defined in Section 1613A.5.2.

SITE COEFFICIENTS. The values of F_a and F_v indicated in Tables 1613A.5.3(1) and 1613A.5.3(2), respectively.

*(Relocated from 1627A, 2001 CBC) **SOIL-STRUCTURE RESONANCE.** The coincidence of the natural period of a structure with a dominant frequency of the ground motion.*

*(Relocated from 1627A, 2001 CBC) **STRUCTURAL ELEMENTS.** Floor or roof diaphragms, decking, joists, slabs, beams, or girders, columns, bearing walls, retaining walls, masonry or concrete nonbearing walls exceeding one story in height, foundations, shear walls or other lateral-force-resisting members, and any other elements necessary to the vertical and lateral strength or stability of either the building as a whole or any of its parts, including connection between such elements.*

1613A.3 Existing buildings. *[For OSHPD 1 & 4]* Additions, alterations, modification, or change of occupancy of existing buildings shall be in accordance with Sections 3403A.2.3 and 3406A.4.

1613A.4 Special inspections. Where required by Section 1705A.3, the statement of special inspections shall include the special inspections required by Section 1705A.3.1.

1613A.5 Seismic ground motion values. Seismic ground motion values shall be determined in accordance with this section.

(Figure 1613.5(1) is stricken.)

FIGURE 1613.5(1) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR THE CONTERMINOUS UNITED STATES OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B

(Figure 1613.5(1) “—continued” is stricken.)

~~FIGURE 1613.5(1)—continued MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR THE CONTERMINOUS UNITED STATES OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(2) is stricken.)

~~FIGURE 1613.5(2) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR THE CONTERMINOUS UNITED STATES OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(2) “—continued” is stricken.)

~~FIGURE 1613.5(2)—continued MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR THE CONTERMINOUS UNITED STATES OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(3) is stricken.)

~~FIGURE 1613.5(3) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 1 OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(3) “—continued” is stricken.)

~~FIGURE 1613.5(3)—continued MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 1 OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(4) is stricken.)

~~FIGURE 1613.5(4) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 1 OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(4) “—continued” is stricken.)

~~FIGURE 1613.5(4) continued MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 1 OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(5) is stricken.)

~~FIGURE 1613.5(5) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 2 OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(6) is stricken.)

~~FIGURE 1613.5(6) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 2 OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(7) is stricken.)

~~FIGURE 1613.5(7) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 3 OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% PERCENT OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(8) is stricken.)

~~FIGURE 1613.5(8) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 3 OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(9) is stricken.)

~~FIGURE 1613.5(9) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR REGION 4 OF 0.2 AND 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(10) is stricken.)

~~FIGURE 1613.5(10) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR HAWAII OF 0.2 AND 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(11) is stricken.)

~~FIGURE 1613.5(11) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR ALASKA OF 0.2 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(12) is stricken.)

~~FIGURE 1613.5(12) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR ALASKA OF 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(13) is stricken.)

~~FIGURE 1613.5(13) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR PUERTO RICO, CULEBRA, VIEQUES, ST. THOMAS, ST. JOHN AND ST. CROIX OF 0.2 AND 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

(Figure 1613.5(14) is stricken.)

~~FIGURE 1613.5(14) MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION FOR GUAM AND TUTUILLA OF 0.2 AND 1.0 SEC SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING), SITE CLASS B~~

1613A.5.1 Mapped acceleration parameters. The parameters S_s and S_1 shall be determined from the 0.2 and 1-second spectral response accelerations shown on Figures 1613.5(1) through 1613.5(14). ~~Where S_1 is less than or equal to 0.04 and S_s is less than or equal to 0.15, the structure is permitted to be assigned to Seismic Design Category A.~~

1613A.5.2 Site class definitions. Based on the site soil properties, the site shall be classified as either Site Class A, B, C, D, E or F in accordance with Table 1613A.5.2. When the soil properties are not known in sufficient detail to determine the site class, Site Class D shall be used unless the building official or geotechnical data determines that Site Class E or F soil is likely to be present at the site.

TABLE 1613A.5.2 - SITE CLASS DEFINITIONS

SITE CLASS	SOIL PROFILE NAME	AVERAGE PROPERTIES IN TOP 100 feet, SEE SECTION 1613A.5.5		
		Soil shear wave velocity, \bar{v}_s , (ft/s)	Standard penetration resistance, \bar{N}	Soil undrained shear strength, \bar{s}_u , (psf)
A	Hard rock	$\bar{v}_s > 5,000$	N/A	N/A
B	Rock	$2,500 < \bar{v}_s \leq 5,000$	N/A	N/A
C	Very dense soil and soft rock	$1,200 < \bar{v}_s \leq 2,500$	$\bar{N} > 50$	$\bar{s}_u \geq 2,000$
D	Stiff soil profile	$600 \leq \bar{v}_s \leq 1,200$	$15 \leq \bar{N} \leq 50$	$1,000 \leq \bar{s}_u \leq 2,000$
E	Soft soil profile	$\bar{v}_s < 600$	$\bar{N} < 15$	$\bar{s}_u < 1,000$
E	—	Any profile with more than 10 feet of soil having the following characteristics: 1. Plasticity index $PI > 20$, 2. Moisture content $w \geq 40\%$, and 3. Undrained shear strength $\bar{s}_u < 500$ psf		
F	—	Any profile containing soils having one or more of the following characteristics: 1. Soils vulnerable to potential failure or collapse under seismic loading such as liquefiable soils, quick and highly sensitive clays, collapsible weakly cemented soils. 2. Peats and/or highly organic clays ($H > 10$ feet of peat and/or highly organic clay where H = thickness of soil) 3. Very high plasticity clays ($H > 25$ feet with plasticity index $PI > 75$) 4. Very thick soft/medium stiff clays ($H > 120$ feet)		

For SI: 1 foot = 304.8 mm, 1 square foot = 0.0929 m², 1 pound per square foot = 0.0479 kPa. N/A = Not applicable

1613A.5.3 Site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The maximum considered earthquake spectral response acceleration for short periods, S_{MS} , and at 1-second period, S_{M1} , adjusted for site class effects shall be determined by Equations 16A-37 and 16A-38, respectively:

$$S_{MS} = F_a S_s$$

(Equation 16A-37)

$$S_{M1} = F_v S_1$$

(Equation 16A-38)

where:

F_a = Site coefficient defined in Table 1613A.5.3(1).

F_v = Site coefficient defined in Table 1613A.5.3(2).

S_s = The mapped spectral accelerations for short periods as determined in Section 1613A.5.1.

S_1 = The mapped spectral accelerations for a 1-second period as determined in Section 1613A.5.1.

TABLE 1613A.5.3(1) - VALUES OF SITE COEFFICIENT F_a ^a

SITE CLASS	MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD				
	$S_s \leq 0.25$	$S_s = 0.50$	$S_s = 0.75$	$S_s = 1.00$	$S_s \geq 1.25$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.2	1.2	1.1	1.0	1.0
D	1.6	1.4	1.2	1.1	1.0
E	2.5	1.7	1.2	0.9	0.9
F	Note b	Note b	Note b	Note b	Note b

a. Use straight-line interpolation for intermediate values of mapped spectral response acceleration at short period, S_s .

b. Values shall be determined in accordance with Section 11.4.7 of ASCE 7.

TABLE 1613A.5.3(2) - VALUES OF SITE COEFFICIENT F_v ^a

SITE CLASS	MAPPED SPECTRAL RESPONSE ACCELERATION AT 1-SECOND PERIOD				
	$S_1 \leq 0.1$	$S_1 = 0.2$	$S_1 = 0.3$	$S_1 = 0.4$	$S_1 \geq 0.5$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.7	1.6	1.5	1.4	1.3
D	2.4	2.0	1.8	1.6	1.5
E	3.5	3.2	2.8	2.4	2.4
F	Note b	Note b	Note b	Note b	Note b

a. Use straight-line interpolation for intermediate values of mapped spectral response acceleration at 1-second period, S_1 .

b. Values shall be determined in accordance with Section 11.4.7 of ASCE 7.

1613A.5.4 Design spectral response acceleration parameters. Five-percent damped design spectral response acceleration at short periods, S_{DS} , and at 1-second period, S_{D1} , shall be determined from Equations 16A-39 and 16A-40, respectively:

$$S_{DS} = \frac{2}{3} S_{MS} \quad \text{(Equation 16A-39)}$$

$$S_{D1} = \frac{2}{3} S_{M1} \quad \text{(Equation 16A-40)}$$

where:

S_{MS} = The maximum considered earthquake spectral response accelerations for short period as determined in Section 1613A.5.3.

S_{M1} = The maximum considered earthquake spectral response accelerations for 1-second period as determined in Section 1613A.5.3.

1613A.5.5 Site classification for seismic design. Site classification for Site Class C, D or E shall be determined from Table 1613A.5.5.

The notations presented below apply to the upper 100 feet (30 480 mm) of the site profile. Profiles containing distinctly different soil and/or rock layers shall be subdivided into those layers designated by a number that ranges from 1 to n at the bottom where there is a total of n distinct layers in the upper 100 feet (30 480 mm). The symbol i then refers to any one of the layers between 1 and n .

where:

v_{si} = The shear wave velocity in feet per second (m/s).

d_i = The thickness of any layer between 0 and 100 feet (30 480 mm).

where:

$$\bar{v}_s = \frac{\sum_{i=1}^n d_i}{\sum_{i=1}^n \frac{d_i}{v_{si}}}$$

(Equation 16A-41)

$$\sum_{i=1}^n d_i = 100 \text{ feet (30 480 mm)}$$

N_i is the Standard Penetration Resistance (ASTM D 1586) not to exceed 100 blows/foot (305 mm) as directly measured in the field without corrections. When refusal is met for a rock layer, N_i shall be taken as 100 blows/foot (305 mm).

$$\bar{N} = \frac{\sum_{i=1}^n d_i}{\sum_{i=1}^n \frac{d_i}{N_i}}$$

(Equation 16A-42)

where N_i and d_i in Equation 16A-42 are for cohesionless soil, cohesive soil and rock layers.

$$\bar{N}_{ch} = \frac{d_s}{\sum_{i=1}^m \frac{d_i}{N_i}}$$

(Equation 16A-43)

where:

$$\sum_{i=1}^m d_i = d_s$$

Use d_i and N_i for cohesionless soil layers only in Equation 16A-43.

d_s = The total thickness of cohesionless soil layers in the top 100 feet (30 480 mm).

m = The number of cohesionless soil layers in the top 100 feet (30 480 mm).

s_{ui} = The undrained shear strength in psf (kPa), not to exceed 5,000 psf (240 kPa), ASTM D 2166 or D 2850.

$$\bar{s}_u = \frac{d_c}{\sum_{i=1}^k \frac{d_i}{s_{ui}}}$$

(Equation 16A-44)

where:

$$\sum_{i=1}^k d_i = d_c$$

d_c = The total thickness of cohesive soil layers in the top 100 feet (30 480 mm).

k = The number of cohesive soil layers in the top 100 feet (30 480 mm).

PI = The plasticity index, ASTM D 4318.

w = The moisture content in percent, ASTM D 2216.

Where a site does not qualify under the criteria for Site Class F and there is a total thickness of soft clay greater than 10 feet (3048 mm) where a soft clay layer is defined by: $s_u < 500$ psf (24 kPa), $w \geq 40$ percent, and $PI > 20$, it shall be classified as Site Class E.

The shear wave velocity for rock, Site Class B, shall be either measured on site or estimated by a geotechnical engineer or engineering geologist/seismologist for competent rock with moderate fracturing and weathering. Softer and more highly fractured and weathered rock shall either be measured on site for shear wave velocity or classified as Site Class C.

The hard rock category, Site Class A, shall be supported by shear wave velocity measurements either on site or on profiles of the same rock type in the same formation with an equal or greater degree of weathering and fracturing. Where hard rock conditions are known to be continuous to a depth of 100 feet (30 480 mm), surficial shear wave velocity measurements are permitted to be extrapolated to assess \bar{v}_s .

The rock categories, Site Classes A and B, shall not be used if there is more than 10 feet (3048 mm) of soil between the rock surface and the bottom of the spread footing or mat foundation.

TABLE 1613A.5.5 - SITE CLASSIFICATION^a

SITE CLASS	\bar{v}_s	\bar{N} or \bar{N}_{ch}	\bar{s}_w
E	< 600 ft/s	< 15	< 1,000 psf
D	600 to 1,200 ft/s	15 to 50	1,000 to 2,000 psf
C	1,200 to 2,500 ft/s	> 50	> 2,000

For SI: 1 foot per second = 304.8 mm per second, 1 pound per square foot = 0.0479 kN/m².

- a. If the \bar{s}_w method is used and the \bar{N}_{ch} and \bar{s}_w criteria differ, select the category with the softer soils (for example, use Site Class E instead of D).

1613A.5.5.1 Steps for classifying a site.

1. Check for the four categories of Site Class F requiring site-specific evaluation. If the site corresponds to any of these categories, classify the site as Site Class F and conduct a site-specific evaluation.
2. Check for the existence of a total thickness of soft clay > 10 feet (3048 mm) where a soft clay layer is defined by: $\bar{s}_w < 500$ psf (24 kPa), $w \geq 40$ percent and $PI > 20$. If these criteria are satisfied, classify the site as Site Class E.
3. Categorize the site using one of the following three methods with \bar{v}_s , \bar{N} , and \bar{s}_w and computed in all cases as specified.
 - 3.1. \bar{v}_s for the top 100 feet (30 480 mm) (\bar{v}_s method).
 - 3.2. \bar{N}_{ch} for the top 100 feet (30 480 mm) (\bar{N} method).
 - 3.3. \bar{N} for cohesionless soil layers ($PI < 20$) in the top 100 feet (30 480 mm) and average, \bar{s}_w for cohesive soil layers ($PI > 20$) in the top 100 feet (30 480 mm) (\bar{s}_w method).

1613A.5.6 Determination of seismic design category. Occupancy Category I, II or III structures located where the mapped spectral response acceleration parameter at 1-second period, S_1 , is greater than or equal to 0.75 shall be assigned to Seismic Design Category E. Occupancy Category IV structures located where the mapped spectral response acceleration parameter at 1-second period, S_1 , is greater than or equal to 0.75 shall be assigned to Seismic Design Category F. All other structures shall be assigned to seismic design category D. a seismic design category based on their occupancy category and the design spectral response acceleration coefficients, S_{DS} and S_{D1} , determined in accordance with Section 1613.5.4 or the site specific procedures of ASCE 7. Each building and structure shall be assigned to the more severe seismic design category in accordance with Table 1613.5.6(1) or 1613.5.6(2), irrespective of the fundamental period of vibration of the structure, T .

TABLE 1613.5.6(1) – SEISMIC DESIGN CATEGORY BASED ON SHORT-PERIOD RESPONSE ACCELERATIONS

VALUE OF S_{DS}	OCCUPANCY CATEGORY		
	I or II	III	IV
$S_{DS} < 0.167g$	A	A	A
$0.167g \leq S_{DS} < 0.33g$	B	B	C
$0.33g \leq S_{DS} < 0.50g$	C	C	D
$0.50g \leq S_{DS}$	D	D	D

TABLE 1613.5.6(2) — SEISMIC DESIGN CATEGORY BASED ON 1-SECOND PERIOD RESPONSE ACCELERATION

VALUE OF S_{D1}	OCCUPANCY CATEGORY		
	I or II	III	IV
$S_{D1} < 0.067g$	A	A	A
$0.067g \leq S_{D1} < 0.133g$	B	B	C
$0.133g \leq S_{D1} < 0.20g$	C	C	D
$0.20g \leq S_{D1}$	D	D	D

1613A.5.6.1 Alternative seismic design category determination. *Not permitted by OSHPD and DSA-SS.* Where S_1 is less than 0.75, the seismic design category is permitted to be determined from Table 1613.5.6(1) alone when all of the following apply:

1. In each of the two orthogonal directions, the approximate fundamental period of the structure, T_a , in each of the two orthogonal directions determined in accordance with Section 12.8.2.1 of ASCE 7, is less than $0.8 T_s$, determined in accordance with Section 11.4.5 of ASCE 7.
2. In each of the two orthogonal directions, the fundamental period of the structure used to calculate the story drift is less than T_s .
3. Equation 12.8-2 of ASCE 7 is used to determine the seismic response coefficient, C_s .
4. The diaphragms are rigid as defined in Section 12.3.1 in ASCE 7 or for diaphragms that are flexible, the distance between vertical elements of the seismic force-resisting system does not exceed 40 feet (12 192 mm).

1613A.5.6.2 (Relocated from 1630A.2.3, 2001 CBC) Simplified design procedure. *Not permitted by OSHPD and DSA-SS.* Where the alternate simplified design procedure of ASCE 7 is used, the seismic design category shall be determined in accordance with ASCE 7.

1613A.6 Alternatives to ASCE 7. The provisions of Section 1613A.6 shall be permitted as alternatives to the relevant provisions of ASCE 7.

1613A.6.1 Assumption of flexible diaphragm. Add the following text at the end of Section 12.3.1.1 of ASCE 7:

Diaphragms constructed of wood structural panels or untopped steel decking shall also be permitted to be idealized as flexible, provided all of the following conditions are met:

1. Toppings of concrete or similar materials are not placed over wood structural panel diaphragms except for nonstructural toppings no greater than $1\frac{1}{2}$ inches (38 mm) thick.
2. Each line of vertical elements of the lateral-force-resisting system complies with the allowable story drift of Table 12.12-1.
3. Vertical elements of the lateral-force-resisting system are light-framed walls sheathed with wood structural panels rated for shear resistance or steel sheets.
4. Portions of wood structural panel diaphragms that cantilever beyond the vertical elements of the lateral-force-resisting system are designed in accordance with Section 2305.2.5 of the *International California Building Code*.

1613A.6.2 Additional seismic-force-resisting systems for seismically isolated structures. Add the following exception to the end of Section 17.5.4.2 of ASCE 7:

Exception: For isolated structures designed in accordance with this standard, the Structural System Limitations and the Building Height Limitations in Table 12.2-1 for ordinary steel concentrically braced frames (OCBFs) as defined in Chapter 11 and ordinary *intermediate* moment frames (OMFs) (*IMFs*) as

defined in Chapter 11 are permitted to be taken as 160 feet (48 768 mm) for structures assigned to Seismic Design Category D, E or F, provided that the following conditions are satisfied:

1. The value of R_1 as defined in Chapter 17 is taken as 1.
2. For ~~OMFs~~ and OCBFs, design is in accordance with AISC 341.
3. For IMFs, design is in accordance with AISC 341. In addition, requirements of Section 9.3 of AISC 341 shall be satisfied.

SECTION 1614A - MODIFICATIONS TO ASCE 7

1614A.1 General. *The text of ASCE 7 shall be modified as indicated in sections 1614A.1.1 through 1614A.1.31.*

1614A.1.1 ASCE 7, Section 11.1. *Modify ASCE 7 Section 11.1 by adding section 11.1.5 as follows:*

11.1.5 Design Criteria Requirements. *Prior to implementation of the non-linear design procedures – the ground motion, analysis and design methods, material assumptions and acceptance criteria proposed by the engineer shall be submitted to the enforcement agency in the form of design criteria for approval.*

The analysis and design basis, conclusion and design decisions shall be reviewed and accepted by the enforcement agent.

1614A.1.2 ASCE 7, Section 11.4.7. *Replace ASCE 7 Section 11.4.7 as follows:*

11.4.7 Site-specific ground motion procedures. *The site-specific ground motion procedure set forth in ASCE 7 Section 21 as modified in Section 1802A.6 of this code are permitted to be used to determine ground motion for any structure.*

Unless otherwise approved, the site-specific procedure per ASCE 7 Section 21 as modified by Section 1802A.6 of this code shall be used where any of the following conditions apply:

- 1) A site response analysis shall be performed per Section 21.1 and a ground motion hazard analysis shall be performed in accordance with Section 21.2 for the following structures:
 - a) Structure located in Type E soils and mapped MCE spectral acceleration at short periods (S_s) exceeds 2.0g.
 - b) Structures located in Type F soils.

Exception:

- 1) Where S_s is less than 0.20g, use of Type E soil profile shall be permitted.
- 2) Where exception to Section 20.3.1 is applicable except for base isolated buildings.

2) A ground motion hazard analysis shall be performed in accordance with Section 21.2 when:

- a) A time history response analysis of the building is performed as part of the design.
- b) The building site is located within 10 kilometers of an active fault.
- c) For seismically isolated structures and for structures with damping systems.

1614A.1.3 ASCE 7, Table 12.2 -1. *Modify ASCE 7 Table 12.2-1 as follows:*

A. BEARING WALL SYSTEMS

14. Light-framed walls with shear panels of all other materials – Not permitted by OSHPD and DSA-SS.

B. BUILDING FRAME SYSTEMS

4. Ordinary steel concentrically braced frames – Not permitted by OSHPD.

24. Light-framed walls with shear panels of all other materials – Not permitted by OSHPD and DSA-SS.

25. Buckling-restrained braced frames, non-moment-resisting beam-column connections – Not permitted by OSHPD.

27. Special steel plate shear wall – Not permitted by OSHPD.

C. MOMENT RESISTING FRAME SYSTEMS

2. Special steel truss moment frames – Not permitted by OSHPD.

3. Intermediate steel moment frames – Not permitted by OSHPD.

4. Ordinary steel moment frames – Not permitted by OSHPD.

Exception:

1) Systems listed in this section can be used as an ~~alternate~~ alternative system when pre-approved by the enforcement agency.

2) Rooftop or other supported structures not exceeding two stories in height and 10 percent of the total structure weight can use the systems in this section when designed as components per ASCE 7 Chapter 13.

3) Systems listed in this section can be used for seismically isolated buildings when permitted by Section 1613A.6.2.

1614A.1.4 ASCE 7, Section 12.2.3.1. Modify ASCE 7 Section 12.2.3.1 by adding the following additional requirements for two stage equivalent lateral force procedure:

(Relocated from 1630A.4.2, 2001 CBC)

~~2. provided that structure complies with the following.~~

e. Where design of elements of the upper portion is governed by special seismic load combinations, the special loads shall be considered in the design of lower portions.

~~2.4 The lower portion shall have a stiffness at least 10 times the upper portion.~~

~~2.5 The period of the entire structure shall not be greater than 1.1 times the period of the upper portion considered as a separate structural system fixed at the base.~~

f. The detailing requirements required for the lateral system of the upper portion shall be used for structural components common to the structural system of lower portion.

g. If separate models are used to design the upper and lower portions, the model boundary conditions of the upper portion shall be compatible with actual strength and stiffness of the supporting elements of the lower portion.

h. *(Relocated from 1629A.8.3, 2001 CBC)* Both flexible upper portion and rigid lower portion considered separately can be classified as being regular.

Exception: When dynamic analysis is used regularity requirements in item # h above need not apply.

1614A.1.5 ASCE 7, Section 12.3.3. *Modify first sentence of ASCE 7 Section 12.3.3.1 as follows:*

(Relocated from 1629A.9.1, 1629A.9.4, 1629A.9.5, 2001 CBC)

12.3.3.1 Prohibited Horizontal and Vertical Irregularities for Seismic Design Categories D through F. *Structures assigned to Seismic Design Category D, E or F having horizontal structural irregularity Type 1b of Table 12.3-1 or vertical structural irregularities Type 1b, 5a or 5b of Table 12.3-2 shall not be permitted.*

1614A.1.6 ASCE 7, Section 12.7.2. *Modify ASCE 7 Section 12.7.2 by adding item 5 to read as follows:*

(Relocated from 1630A.1.1, Item 5, 2001 CBC)

5. *Where buildings provide lateral support for walls retaining earth, and the exterior grades on opposite sides of the building differ by more than 6 feet (1829 mm), the load combination of the seismic increment of earth pressure due to earthquake acting on the higher side, as determined by a ~~civil~~ Geotechnical engineer qualified in soils engineering plus the difference in earth pressures shall be added to the lateral forces provided in this section.*

1614A.1.7 ASCE 7, Section 12.8.1.1. *Modify ASCE 7 Section 12.8.1.1 by replacing equation 12.8-5 as follows:*

$$C_s = 0.03 \quad (12.8-5)$$

1614A.1.8 ASCE 7, Section 12.8.7. *Modify ASCE 7 Section 12.8.7 by replacing equation 12.8-16 as follows:*

$$\theta = \frac{P_x \Delta I}{V_x h_{sx} C_d} \quad (12.8-16)$$

1614A.1.9 ASCE 7, Section 12.9.4. *Replace ASCE 7 Section 12.9.4 as follows:*

(Relocated from 1631A.5.4, 2001 CBC)

12.9.4 Scaling Design Values of Combined Response. *Modal base shear shall not be less than the base shear calculated using the equivalent lateral force procedure of section 12.8. ~~, except where the fundamental period exceeds $(C_u)(T_u)$, then $(C_u)(T_u)$ shall be used in lieu of T for fundamental period in calculating equivalent static base shear.~~*

1614A.1.10 ASCE 7, Section 12.13.1. *Modify ASCE 7 section 12.13.1 by adding Section 12.13.1.1 as follows:*

(Relocated from 1633A.2.12, 2001 CBC)

12.13.1.1 Foundations and superstructure-to-foundation connections. *The foundation shall be capable of transmitting the design base shear and the overturning forces from the structure into the supporting soil.*

In addition, the foundation and the connection of the superstructure elements to the foundation shall have the strength to resist, in addition to gravity loads, the lesser of the following seismic loads:

- 1. The strength of the superstructure elements.*
- 2. The maximum forces that would occur in the fully yielded structural system.*
- 3. ~~No times the forces in the superstructure elements due to the seismic forces as prescribed in this chapter.~~ Forces from Load Combinations with overstrength factor per ASCE 7 Section 12.4.3.2.*

EXCEPTIONS:

1. Where structures are designed using $R \leq 2.5$ such as for inverted pendulum-type structures.
2. When it can be demonstrated that inelastic deformation of the foundation and superstructure-to-foundation connection will not result in a weak story or cause collapse of the structure.
3. Where basic structural system consists of light framed walls with shear panels.

Where the computation of the seismic overturning moment is by the equivalent lateral-force method or the modal analysis method, reduction in overturning moment permitted by section 12.13.4 of ASCE 7 may be used.

Where moment resistance is assumed at the base of the superstructure elements, the rotation and flexural deformation of the foundation as well as deformation of the superstructure-to-foundation connection shall be considered in the drift and deformation compatibility analyses.

Exception: The seismic loads defined above need not be considered for friction and passive resistance. Ultimate soil pressure can be used when considering load combinations with the seismic loads defined above.

1614A.1.11 ASCE 7, Section 13.3.2. Modify ASCE 7 section 13.3.2 by adding the following:

The seismic relative displacements to be used in design of displacement sensitive nonstructural components is $D_p I$ instead of D_p , where D_p is given by equations 13.3-5 to 13.3-8 and I is the building importance factor given in Section 11.5.

1614.1.12 ASCE 7, Section 13.5.6.2. Modify ASCE 7, Section 13.5.6.2 by adding Section 13.5.6.2.3 as follows:

13.5.6.2.3 Additional Requirements

(Relocated from 2501A.5.2, 2001 CBC)

1. **Exitways.** Lay-in ceiling assemblies in exitways of hospitals and essential services buildings shall be installed with a main runner or cross runner surrounding all sides of each piece of tile, board or panel and each light fixture or grille. A cross runner that supports another cross runner shall be considered as a main runner for the purpose of structural classification. Splices or intersections of such runners shall be attached with through connectors such as pop rivets, screws, pins, plates with end tabs or other approved connectors.

(Relocated from 2501A.5.2, 2001 CBC)

2. **Corridors and Lobbies.** Expansion joints shall be provided in the ceiling at intersections of corridors and at junctions of corridors and lobbies or other similar areas.

(Relocated from 2501A.5.4.3, 2001 CBC)

3. **Lay-in panels.** Metal panels and panels weighing more than 1/2 pounds per square foot (24 N/m^2) other than acoustical tiles shall be positively attached to the ceiling suspension runners.

(Relocated from 2501A.5.6.1, 2001 CBC)

4. **Grid members, connectors and expansion devices.** The allowable load-carrying capacity as determined by test shall not exceed one third of the mean ultimate test value based on tests of no fewer than three identical specimens. Rational analysis can be substituted for test where permitted by ASCE 7 and the enforcement agency.

(Relocated from 2501A.5.7.1, 2001 CBC)

5. **Vertical hangers.** Each vertical hanger shall be attached to the ceiling suspension member and to the support above with a minimum of three tight twists in 1-1/2 inches.

(Relocated from 2501A.5.7.2, 2001 CBC)

6a. [For OSHPD 1 & 4] Lateral force bracing. Substantiating design calculations or test reports shall be provided for all lateral force bracing, their connections, and anchorages. Lateral forces must comply with the seismic force requirements of ASCE 7, Chapter 13. Horizontal restraint points shall not be placed more than 8 feet X 12 feet (2438 mm X 3658 mm) on centers. Horizontal restraint wires shall be No. 12 gage minimum and secured to main runners with four tight twists in 1-1/2 inches.

(Relocated from 2501A.5.7.2, 2001 CBC)

6b. [For DSA-SS] Lateral force bracing. Substantiating design calculations or test reports shall be provided for all lateral force bracing, their connections, and anchorages. Lateral forces must comply with the seismic force requirements of ASCE 7, Chapter 13. Horizontal restraint points shall not be placed more than 12 feet X 12 feet (3658 mm X 3658 mm) on centers. Horizontal restraint wires shall be No. 12 gage minimum and secured to main runners with four tight twists in 1-1/2 inches.

(Relocated from 2501A.5.4.2, 2001 CBC)

7. Ceiling fixtures. Fixtures installed in acoustical tile or lay-in panel ceilings shall be mounted in a manner that will not compromise ceiling performance.

All recessed or drop-in light fixtures and grilles shall be supported directly from the fixture housing to the structure above with a minimum of two 12 gage wires located at diagonally opposite corners. Leveling and positioning of fixtures may be provided by the ceiling grid. Fixture support wires may be slightly loose to allow the fixture to seat in the grid system. Fixtures shall not be supported from main runners or cross runners if the weight of the fixtures causes the total dead load to exceed the deflection capability of the ceiling suspension system.

Fixtures shall not be installed so that the main runners or cross runners will be eccentrically loaded.

Surface-mounted fixtures shall be attached to the main runner with at least two positive clamping devices made of material with a minimum of 14 gage. Rotational spring catches do not comply. A 12 gage suspension wire shall be attached to each clamping device and to the structure above.

(Relocated from 2501A.5.9, 2001 CBC)

8. Mechanical services. Terminals and services weighing no more than 20 pounds (9 kg) shall have two no. 12 gage hangers from the terminal or service to the structure above. These wires may be slack.

(Relocated from 2501A.5.8, 2001 CBC)

9. Lighting fixtures. All lighting fixtures shall be positively attached to the suspended ceiling system. The attachment device shall have a capacity of 100 percent of the lighting fixture weight acting in any direction.

Lighting fixtures weighing 56 pounds (25 kg) or more shall be supported directly from the structure above by approved hangers. In such cases the slack wires required by item # 7 above may be omitted.

(Relocated from 2501A.5.10, 2001 CBC)

10. Partitions. Where the suspended ceiling system is required to provide lateral support for the permanent or relocatable partitions, the connection of the partition to the ceiling system, the ceiling system members and their connections, and the lateral force bracing shall be designed to support the reaction force of the partition from prescribed loads applied perpendicular to the face of the partition. These partition reaction forces shall be in addition to the loads described in item #6 above. Partition connectors, the suspended ceiling system and the lateral-force bracing shall all be engineered to suit the individual partition application and shall be shown or defined in the drawings or specifications.

(Relocated from 2501A.5, 2001 CBC)

11. Construction Documents: The construction documents shall include detailing and specifications for suspended ceiling members, connections, support systems, light fixture and mechanical fixture attachments, partition supports and seismic bracing.

1614A.1.13 ASCE 7, Section 13.6.1. Modify ASCE 7 section 13.6.1 by adding Sections 13.6.1.1 & 13.6.1.2 as followings:

13.6.1.1 (Relocated from 1632A.6, 2001 CBC) HVAC Ductwork, Plumbing / Piping and Conduit Systems. Ductwork shall be constructed in accordance with provisions contained in Part 4, Title 24, California Mechanical Code. Where possible, pipes, conduit, and their connections shall be constructed of ductile materials (copper, ductile iron, steel or aluminum and brazed, welded or screwed connections). Pipes, conduits and their connections, constructed of nonductile materials (e.g., cast iron, no-hub pipe and plastic), shall have the brace spacing reduced to satisfy requirements of ASCE 7 Section 13 and not to exceed one-half of the spacing allowed for ductile materials.

13.6.1.2 (Relocated from 1632A.6.1, 2001 CBC) Trapeze Assemblies. All trapeze assemblies supporting pipes, ducts and conduit shall be braced to resist the forces and ~~of Section 1632A.2~~ relative displacements per ASCE 7 Section 13, considering the total weight of the elements on the trapeze.

Pipes, ducts and conduit supported by a trapeze where none of those elements would individually be braced need not be braced if connections to the pipe/conduit/ductwork or directional changes do not restrict the movement of the trapeze. If this flexibility is not provided, bracing will be required when the aggregate weight of the pipes and conduit exceed 10 pounds/feet (146 N/m). The weight shall be determined assuming all pipes and conduit are filled with water.

1614A.1.14 ASCE 7, Section 13.6.7. Modify ASCE 7 Section 13.6.7 by the following:

Requirements of this section shall also apply for $I_p = 1.5$.

1614A.1.15 ASCE 7, Section 13.6.10.1. Modify ASCE 7 Section 13.6.10.1 by adding Section 13.6.10.1.1 as follows:

(Relocated from 1633A.2.13.1, 2001 CBC)
13.6.10.1.1 Elevators guide rail support. The design of guide rail support-bracket fastenings and the supporting structural framing shall ~~be in accordance with Section 3030 (k), Part 7, Title 24,~~ using ~~use~~ the weight of the counterweight or maximum weight of the car plus not more than 40 percent of its rated load. The seismic forces shall be assumed to be distributed one third to the top guiding members and two thirds to the bottom guiding members of cars and counterweights, unless other substantiating data are provided. In addition to the requirements of ASCE 7 Section 13.6.10.1, the minimum seismic forces shall be 0.5g acting in any horizontal direction using allowable stress design.

1614A.1.16 ASCE 7, Section 13.6.10.4. Replace ASCE 7, Section 13.6.10.4 as follows:

(Relocated from 1633A.2.13.1, 2001 CBC)
13.6.10.4 Retainer plates. ~~Retainer plates are required for both car and counterweight, designed in accordance with Section 3032 (c), Part 7, Title 24, California Code of Regulations.~~ Retainer plates are required at the top and bottom of the car and counterweight, except where safety devices acceptable to the enforcement agency are provided which meet all requirements of the retainer plates, including full engagement of the machined portion of the rail. The design of the car, cab stabilizers, counterweight guide rails and counterweight frames for seismic forces shall be based on the following requirements:

1. ~~The lateral forces shall be based on horizontal acceleration of 0.5g for all buildings. The seismic force shall be computed per the requirements of ASCE 7 13.6.10.1. The minimum horizontal acceleration shall be 0.5g for all buildings.~~
2. W_p shall equal the weight of the counterweight or the maximum weight of the car plus not less than 40 percent of its rated load.

3. With the car or counterweight located in the most adverse position, the stress in the rail shall not exceed the limitations specified in these regulations, nor shall the deflection of the rail relative to its supports exceed the deflection listed below:

RAIL SIZE (weight per foot of length, pounds)	WIDTH OF MACHINED SURFACE (inches)	ALLOWABLE RAIL DEFLECTION (inches)
8	1 ¼	0.20
11	1 ½	0.30
12	1 ¾	0.40
15	1 31/32	0.50
18 ½	1 31/32	0.50
22 ½	2	0.50
30	2 ¼	0.50

For SI: 1 inch = 25 mm, 1 foot = 305 mm.

NOTE: Deflection limitations are given to maintain a consistent factor of safety against disengagement of retainer plates from the guide rails during an earthquake.

4. Where guide rails are continuous over supports and rail joints are within 2 feet (610 mm) of their supporting brackets, a simple span may be assumed.
5. The use of spreader brackets is allowed.
6. Cab stabilizers and counterweight frames shall be designed to withstand computed lateral load equal to with a minimum horizontal acceleration of 0.5g using allowable stress design.

1614A.1.17 ASCE 7, Section 15.4.1. Modify ASCE 7, Section 15.4.1 by replacing equations 15.4-1 & 15.4-3 as follows:

$$\frac{C_s = 0.17}{C_s = 0.06}$$

$$\frac{(15.4-1)}{(15.4-3)}$$

1614A.1.18 ASCE 7, Section 17.2.1. Modify ASCE 7, Section 17.2.1 by adding the following:

(Relocated from 1657A.3, 2001 CBC) The importance factor, I_p , for parts and portions of a seismic-isolated building shall be the same as that required for a fixed-base building of the same occupancy category.

1614A.1.19 ASCE 7 Section 17.2.4.7. Modify ASCE 7, Section 17.2.4.7 by adding the following:

(Relocated from 1661A.2.7, 2001 CBC) The effects of uplift and / or rocking shall be explicitly accounted for in the analysis and in the testing of the isolator units.

1614A.1.20 ASCE 7, Section 17.2.4.8. Modify ASCE 7, Section 17.2.4.8 by adding the following:

(Relocated from 1661A.2.8, Item #3, 2001 CBC)

f. ~~These~~ Inspection and replacement programs shall be submitted to enforcement agency for approval with the plans and specifications and shall be a condition of occupancy for the structure.

(Relocated from 1661A.2.8, Item #6, 2001 CBC)

g. After every significant seismic event, the owner shall retain a structural engineer to make an inspection of the structural system. The inspection shall consist of viewing the performance of the building, reviewing the strong motion records, and a visual examination of the isolators and their connections for deterioration, offset or physical damage. A report for each inspection, including conclusions on the continuing adequacy of the structural system, shall be submitted as required to the enforcement agency.

1614A.1.21 ASCE 7, Section 17.2.4.9. Modify ASCE 7, Section 17.2.4.9 by adding the following:

(Relocated from 1661A.2.9, 2001 CBC) The quality control testing program shall include provisions for both prototype and production isolator units. Quality control testing program shall be subject to pre-approval by the enforcement agency.

1614A.1.22 ASCE 7, Section 17.2.4.8. Modify ASCE 7, section 17.2.4.8 by adding section 17.2.4.10 as follows:

(Relocated from 1661A.2.8, Item # 5, 2001 CBC) **17.2.4.10 Instrumentation.** A proposal for instrumentation and equipment specifications shall be forwarded to the enforcement agency for approval.

There shall be sufficient numbers of instruments to characterize the response of the building during an earthquake. Motion measuring instruments shall be located within the building and at levels immediately above and below the isolators. The owner of the building is responsible for the implementation of the instrumentation program. Maintenance of the instrumentation and removal and processing of the records shall be the responsibility of the enforcement agency or its designated agent.

1614A.1.23 ASCE 7, Section 17.2.5.2. Modify ASCE 7, Section 17.2.5.2 by adding the following:

(Relocated from 1661A.3.2, 2001 CBC) The separation requirements for the building above the isolation system and adjacent buildings shall be the sum of the factored displacements for each building. The factors to be used in determining separations shall be:

~~For elastic deformations resulting from the dynamic analysis using the Maximum Capable Earthquake unmodified by R_i or $0.7R$ times the elastic deformations of an adjacent fixed base structure resulting from an equivalent static analysis.~~

1. For seismically isolated buildings, the elastic deformation resulting from the dynamic analyses using the Maximum Capable Earthquake unmodified by R_i .
2. For fixed based buildings, C_d times the elastic deformations resulting from an equivalent static analysis using the seismic base shear computed via ASCE 7 Section 12.8.

1614A.1.24 ASCE 7, Section 17.3.1. Modify ASCE 7, Section 17.3.1 by the adding following:.

(Relocated from 1657A.5.3, Item #3, 2001 CBC) Site-specific ground motion spectra of the design-basis earthquake and the maximum considered earthquake, developed in accordance with Section and 1637A-1802A.6 and ASCE 7, shall be used for design and analysis of all seismic-isolated structures when required by Section 1614A.1.2 or ASCE 7.

1614A.1.25 ASCE 7, Section 17.3.2. Modify ASCE 7, Section 17.3.2 by adding the following:.

(Relocated from 1659A.4.2, 2001 CBC) The SRSS of the time history components shall be equal to or greater than the 5 percent damped design spectra at the isolated period T_i , either T_d or T_m between $0.5T_D$ and $1.25T_M$ (where T_D and T_M are defined in ASCE 7 Section 17.5.3).

~~The duration of the time histories shall be consistent with the magnitude and source characteristics of the design ~~basis~~ earthquake (or maximum ~~capable~~ considered earthquake).~~

~~Time histories developed for sites with a Near-Source Factor, N_s , greater than 1.0 shall incorporate near-source phenomena.~~

1614A.1.26 ASCE 7, Section 17.4.1. Modify ASCE 7, Section 17.4.1 by adding the following:.

(Relocated from 1657A.5.2, 2001 CBC) Equivalent Lateral Force Procedure of Section 17.5 shall be used to establish minimum criteria only, and not be used for design purposes unless these minimum requirements exceed computed force and displacement calculated values from the dynamic analysis.

1614A.1.27 ASCE 7, Section 17.4.2.1. Modify ASCE 7, Section 17.4.2.1 by adding the following.

(Relocated from 1657A.5.3, Item # 1.2.3, 2001 CBC)

3. The isolation system has force-deflection properties that are independent of the rate of loading.

(Relocated from 1657A.5.3, Item # 1.2.4, 2001 CBC)

4. The isolation system has force-deflection properties that are independent of the vertical load and or bilateral load imposed on the isolators.

1614A.1.28 ASCE 7, Section 17.4. Modify ASCE 7, Section 17.4 by adding section 17.4.3 as follows:

(Relocated from 1657A.5.1.1, 2001 CBC)

17.4.3 Period Separation. In each principal direction, the fundamental period, T of the superstructure, computed in accordance with ~~Formula (30-10)~~ ASCE 7 Section 12.8.2, assuming that the structure is fixed at the isolation interface, shall not exceed the isolated-structure period, T_M .

1614A.1.29 ASCE 7, Section 17.4. Modify ASCE 7, Section 17.7 by adding section 17.7.1 as follows:

(Relocated from 1664A.1, 2001 CBC)

17.7.1 Design Review. The design review shall be the responsibility of the enforcement agency. The enforcement agency may at its discretion require the owner of the facility to retain an independent team to review and report ~~on the isolation system design, site conditions and/or building configurations, per Section 17.7.~~ The team shall serve in an advisory capacity to provide technical evaluations to the enforcement agency. The members of the independent team shall be approved by the enforcement agency.

1614A.1.30 ASCE 7, Section 18.2.4. Modify ASCE 7, Section 18.2.4, second sentence as follows:

Regardless of the analysis method used, the peak dynamic response of the structure and elements of the damping system shall be confirmed by using the nonlinear response history procedure.

1614A.1.31 ASCE 7, Section 18.9.2. Modify ASCE 7, Section 18.9.2 by adding the following:

Required Tests of Energy Dissipation Devices - Production Tests. Production testing and associated acceptance criteria shall be as approved by the enforcement agent.

Notation [For DSA-SS]:

Authority: Education Code Sections 17310, 81142; Health & Safety Code Section 16022

Reference(s): Education Code Sections 17280 - 17317, and 811130 - 81149; Health & Safety Code Sections 16000 – 16023

Notation [For OSHPD]:

Authority: Health and Safety Code Section 129850

Reference: Health and Safety Code Sections 1275, 129850 and 129790